VA

U.S. Department of Veterans Affairs Veterans Health Administration

PROJECT NAME:

CONSTRUCT NEW SPS

SITE ADDRESS:

2501 WEST 22ND STREET. SIOUX FALLS, SD 57105

VA PROJECT NUMBER:

438-460

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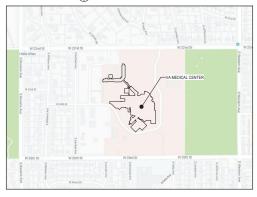
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OWNER AND CONSULTANTS:

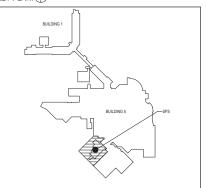
VA COR:		ARCHITECT:		STRUCTURAL E	NGNEER:	
CONTACT:	Brooke White	NAME:	ANDERSON ENGINEERING OF MN, LLC	NAME:	IMEG Corp.	
	2501 WEST 22ND STREET, BUILDING 17 SIOUX FALLS, SD 57105	ADDRESS:	13605 1ST AVE NORTH, SUITE 100 PLYMOUTH, MN 55441	ADDRESS:	1410 WEST RUSSELL STREET SIOUX FALLS, SD 57104	
		CONTACT:	Giovanni Barbari	CONTACT:	Michael Merron	
PHONE:	605.336.3230 EX 7693	PHONE:	763-412-4000	PHONE:	605.331.2505	
		CIVIL ENGINEEP	e e e e e e e e e e e e e e e e e e e	MECH/ELEC/PLUMB/FIRE ENGINEER:		
		NAME:	ANDERSON ENGINEERING OF MN, LLC	NAME:	IMEG Corp.	
		ADDRESS:	13605 1ST AVE NORTH, SUITE 100 PLYMOUTH, MN 55441	ADDRESS:	2882 106TH STREET DES MOINES, IA 50322	
		CONTACT:	EDWIN BRODMARKLE	CONTACT:	Eric Henderson	
		PHONE:	763-412-4000	PHONE:	630.717.2433	
		LANDSCAPE AR	CHITECTURE:			
		NAME:	ANDERSON ENGINEERING OF MN, LLC			
		ADDRESS:	13605 1ST AVE NORTH, SUITE 100 PLYMOUTH, MN 55441			
		CONTACT:	CURT CLAEYS			
		PHONE:	763-412-4000			



SITE LOCATION MAP:



KEY PLAN:

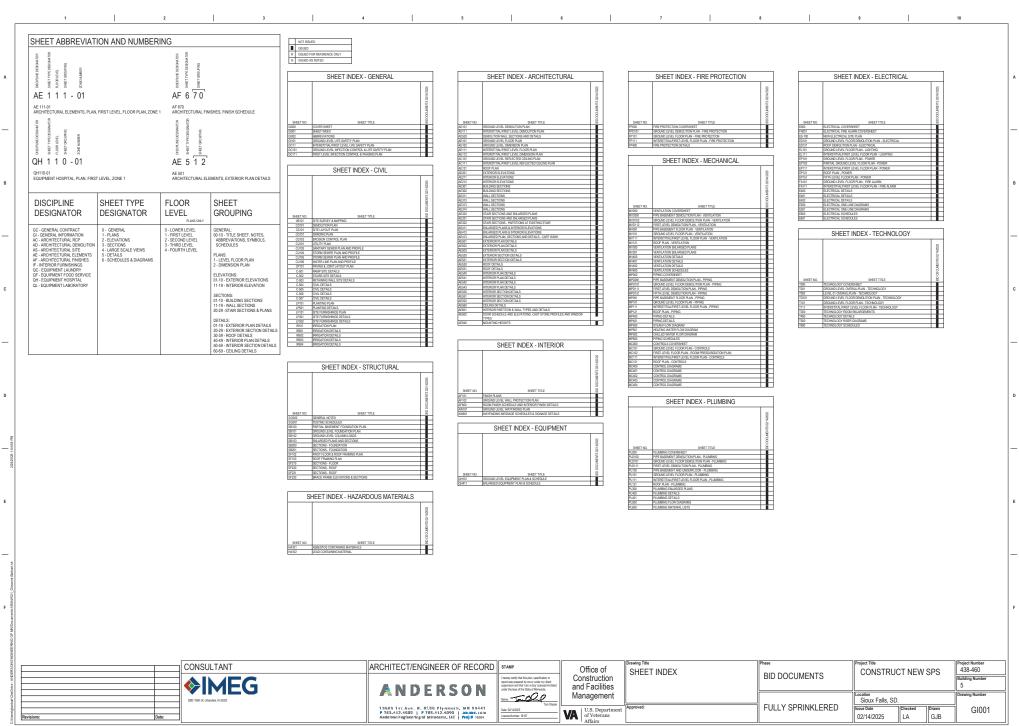


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		CONSULTANT	ARCHITECT/ENGINEER OF RECO	D STAMP	Office of	Drawing Title COVER SHEET	BID DOCUMENTS	Project Title CONSTRUCT NEW SPS	Project Number 438-460
			ANDERSON	I hereby certify that this plan, specification or report was prepared by me or under my direct supervision and that I am a duly Licensed Architect under the laws of the State of Memorsola.	Construction and Facilities		BID DOCOMENTS		Building Number 5
			ANDERSON	here Fall	Management	Approved:		Location Sioux Falls, SD.	Drawing Number
			18605 1at Ave. B. 5180 Figmouth, MR 95441 F 763,412,4080 F 769,412,4090 26-466,con Aederson Registering of Minetania, LK Proj 5 16584	Data 2014/2007	VA U.S. Department of Veterans		FULLY SPRINKLERED	Issue Date Checked Drawn 02/14/2025 LA GJB	G1000
Revisions:	Date:	1	Autor room Fighteering or autoretato, LAC [Proj # 16554		Affairs			02/14/2023	



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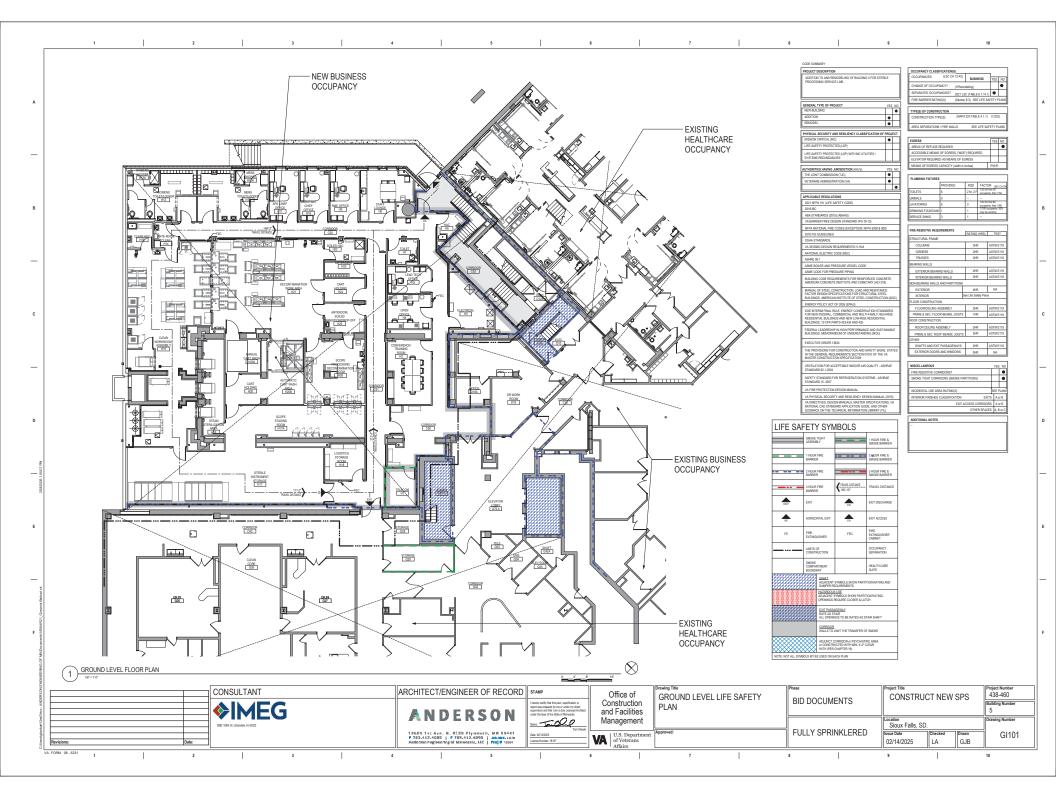
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	WS AMERICAN WELDING SOCIETY PL BASE PLATE AT BATTEN BOARD DOT BOARD FEET (FOOT) BEVEL BEVEL HIMA BUILDER'S HARDWARE MANUFACTURER'S ASSOCIATION	DOC DOCUMENT DR DOCR DRANL DRESSING ROOM, OR DRIVE DR CL DOCR CLOSER DR FR DOCR FRAME DR OPNG DOCR POENING DS DOWNSPOUT DS DO DOSSIN	GA GAGE OF GAL GALLON GALV GALVANI GALV STL GALVANI GB GRAB BA GC GENERAI GD GUARD	(GYPSUM ASSOCIATION C OR GALVANIZED ZED STEEL R . CONTRACTOR	MACH RM MA MAINT MA MAN MA MATL MA MATV MA MAX MA	ICHINE ROOM INITENANCE INIJAL ITERIAL STER ANTENNA TELEVISION SYSTEM XXMUM RKER BOARD	PEN PEND PERF PERIM	PORTLAND CEMENT PLASTER PRIVACY CURTAIN TRACK PEDESTAL PENETRATE PENDANT PERFORATED PERFORATED PERMETER	SHR HD SHRD SHT SHTHG SHV SIM SJ	SHOWER HEAD SHOWER DRAIN SHEET SHEATHING SHELVING SMILAR SCORED JUINT	WFR WOOD FRAME WFS WOOD FURRING STRIPS WH WATER HEATER WHSE WAREHOUSE WP WATERPROOFING WFM WATERPROOFING WFM WATERPROOFING WFM WATERPROOFING WFM WEATHER RESISTANT	
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	28 CATCH BASIN OR CORNER BEAD 295 CENEWTTONS (BACKER) BOARD 200 CONSTRUCTION CHANGE DIRECTIVE CONSTRUCTION CHANGE DIRECTIVE CLOSED ORCUIT TELEVISION CONSTRUCTION DOCUMENTS OR CONTRACT DOCUMENTS 201 CENEVT	EPO EMERGENCY POWER OFF EPS EXPANDED POLYSTYRENE BOARD (INSULATION EQ EQUAL EQUSP EQUALY SPACED EQUIP EQUIPMENT EQUIV EQUIVALENT ERO EXISTING ROF DRAIN	GYP BD GYPSUM GYP PLAS GYPSUM H HIGH OR HAZ MAT HAZARDU HB HOSE BIE HC HOLLOW HCWD HOLLOW HDBD HARDBO.	PLASTER HUMIDISTAT JUS MATERIALS 88 CORE CORE WOOD DOOR ARD	MIR MI MISC MIS MIT MI MKR MA ML ME MLDG MC MLWK MI	RROR SCELLANEOUS TER RRER TALLATH DLDING (MOULDING) LUWORK	PREP PRESS PREV PRKG PRMLD PROJ PROP	PREPARATION PRESSURE PRESUDUS PARKING PREMOLDED PROJECT PROPERTY	STL JST STL LNTL STL PL STL RF DK STL TB STL TR STNLS	STEEL JOIST STEEL INTEL STEEL PLATE STEEL ROOF DECK STEEL TUBE STEEL TRUSS STAINLESS		
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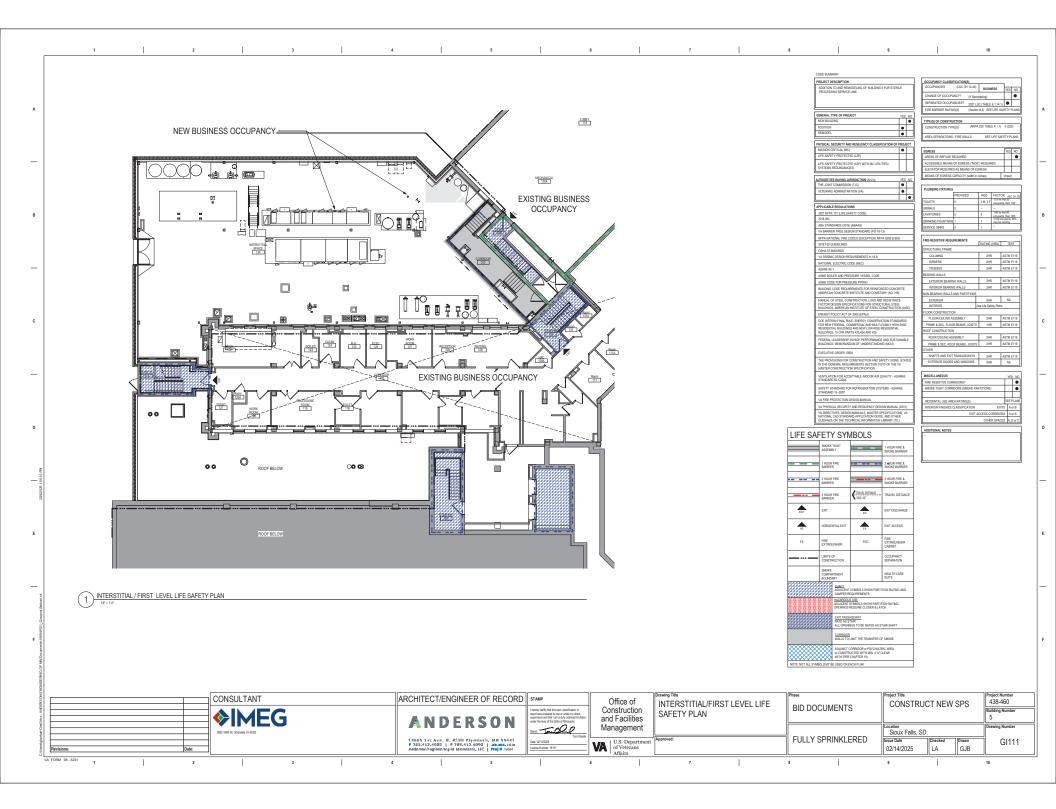
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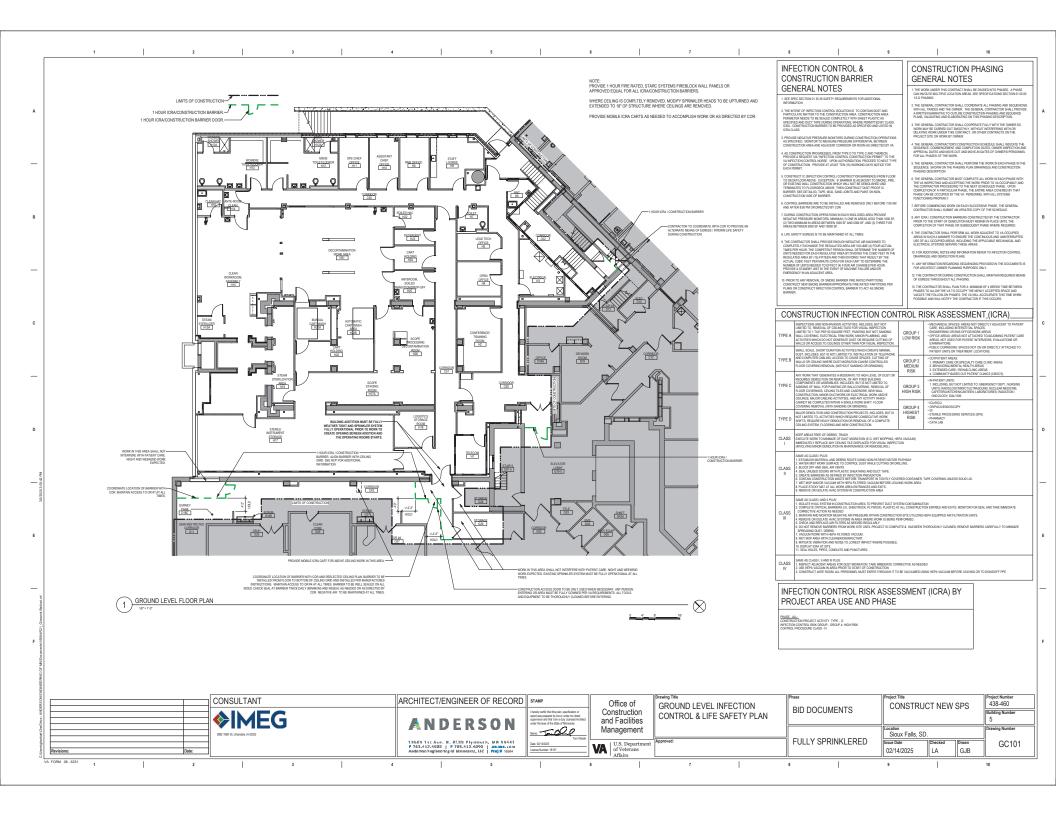
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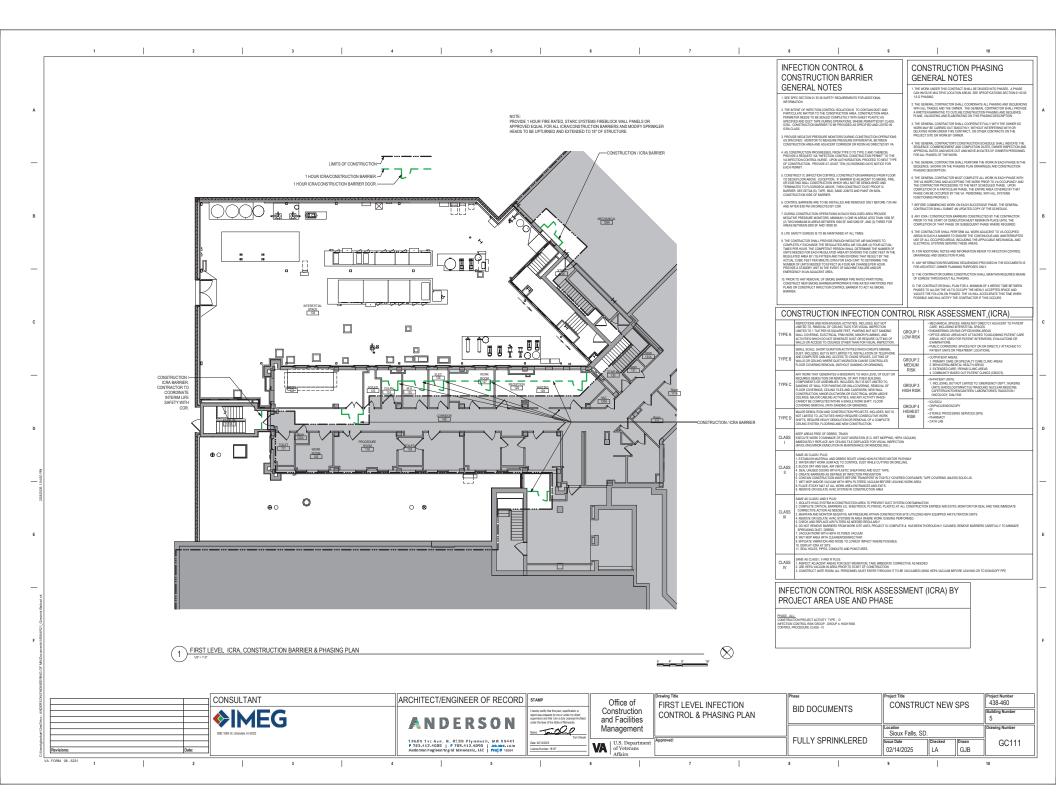
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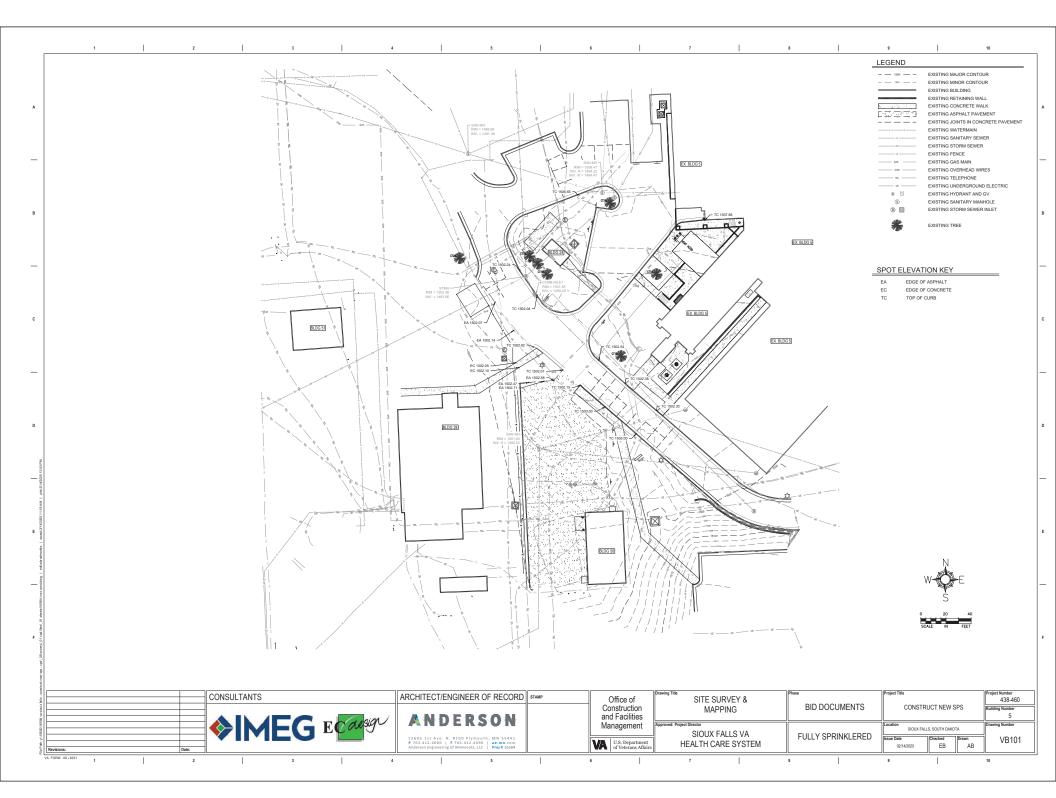
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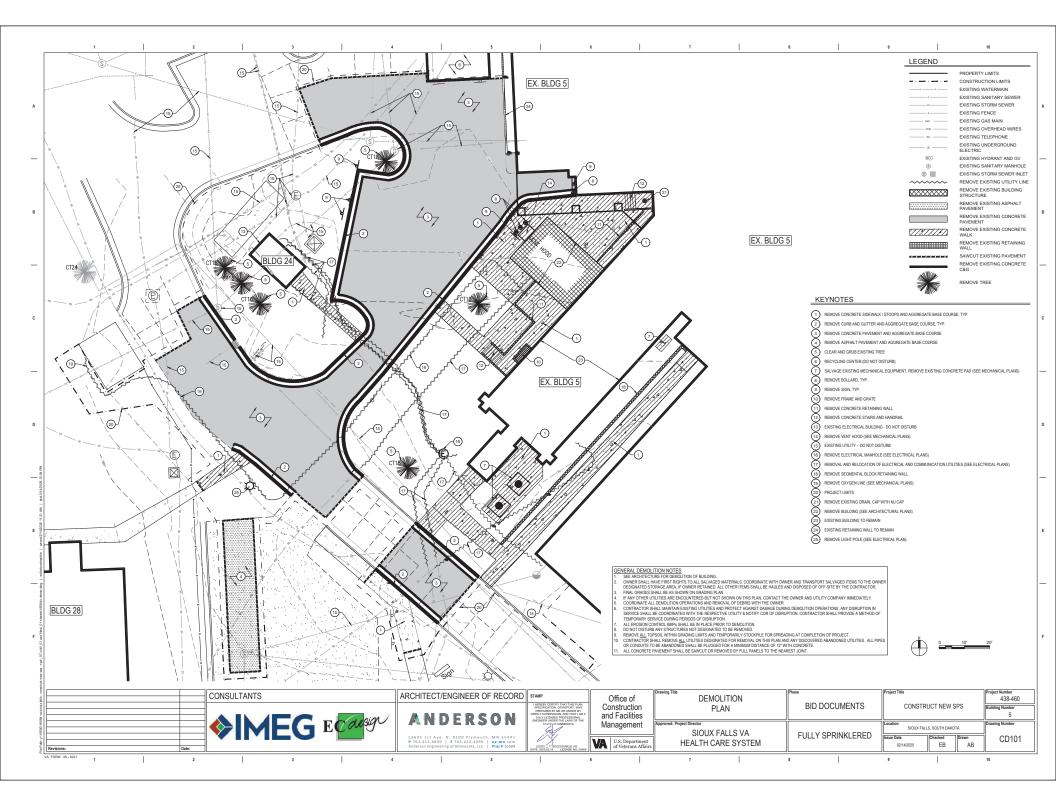


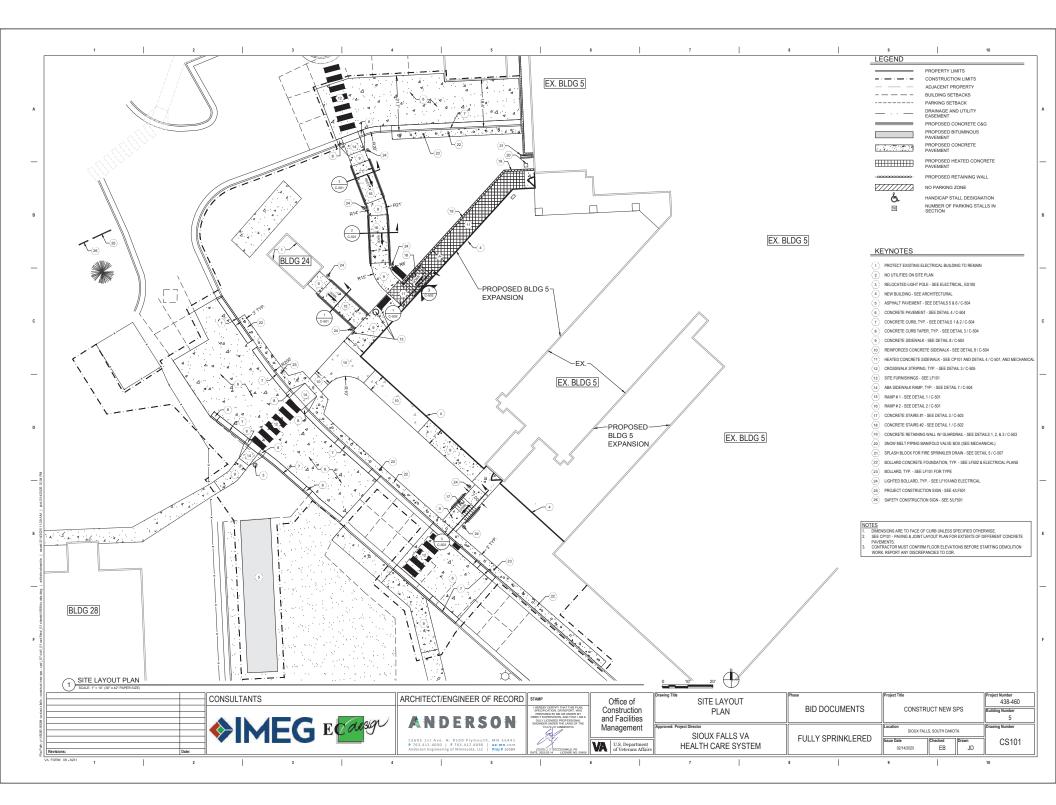


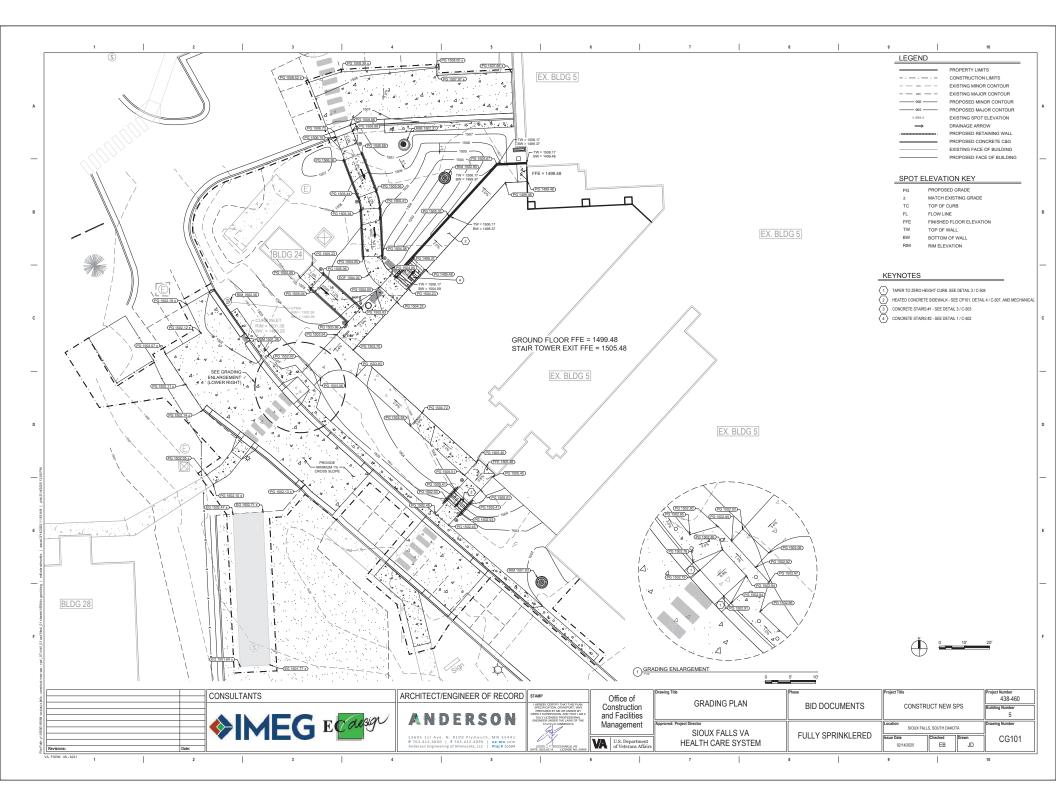


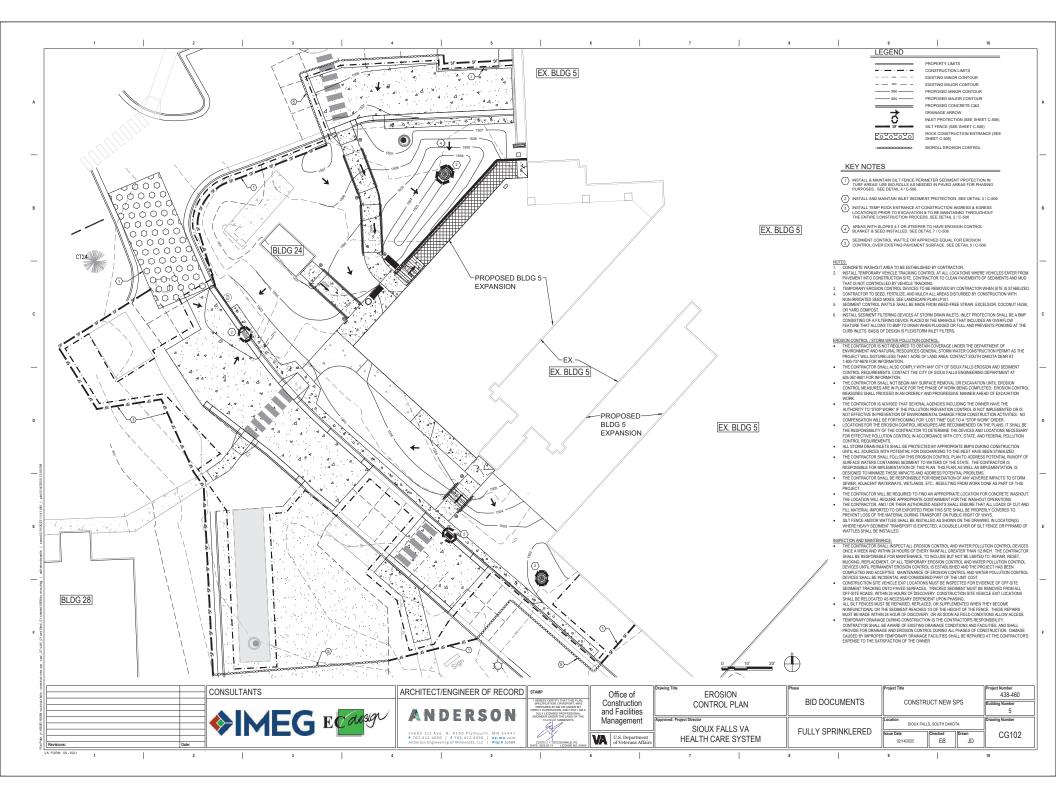


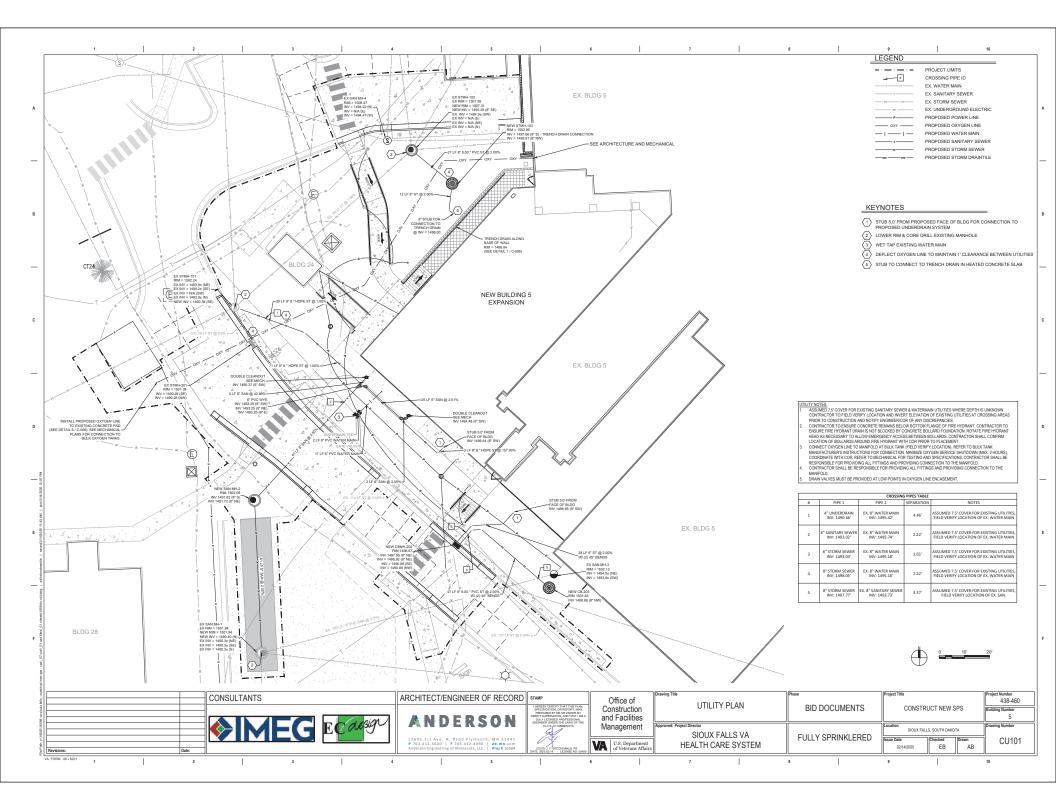


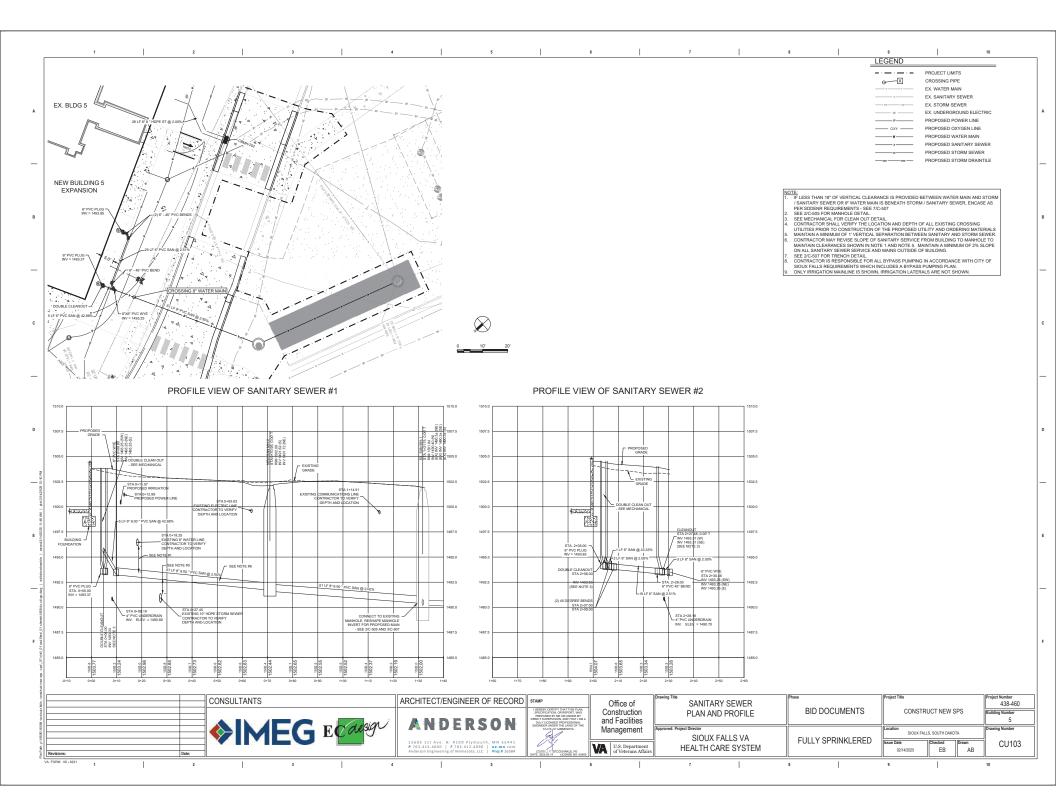


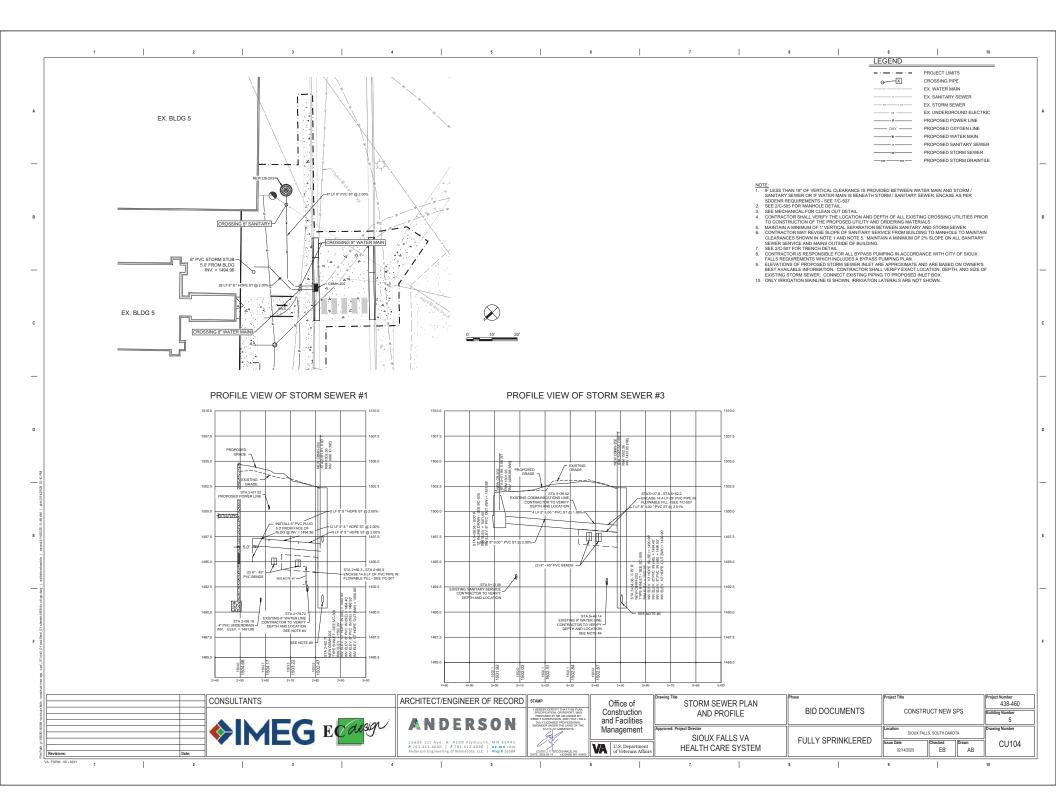


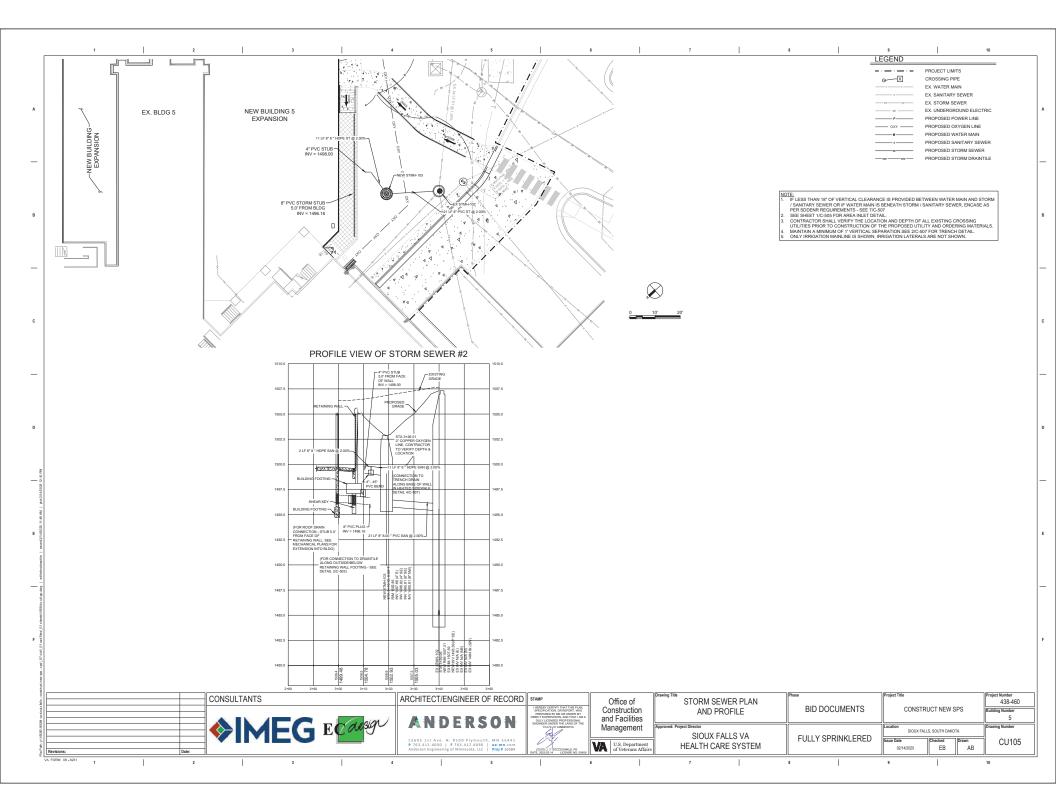


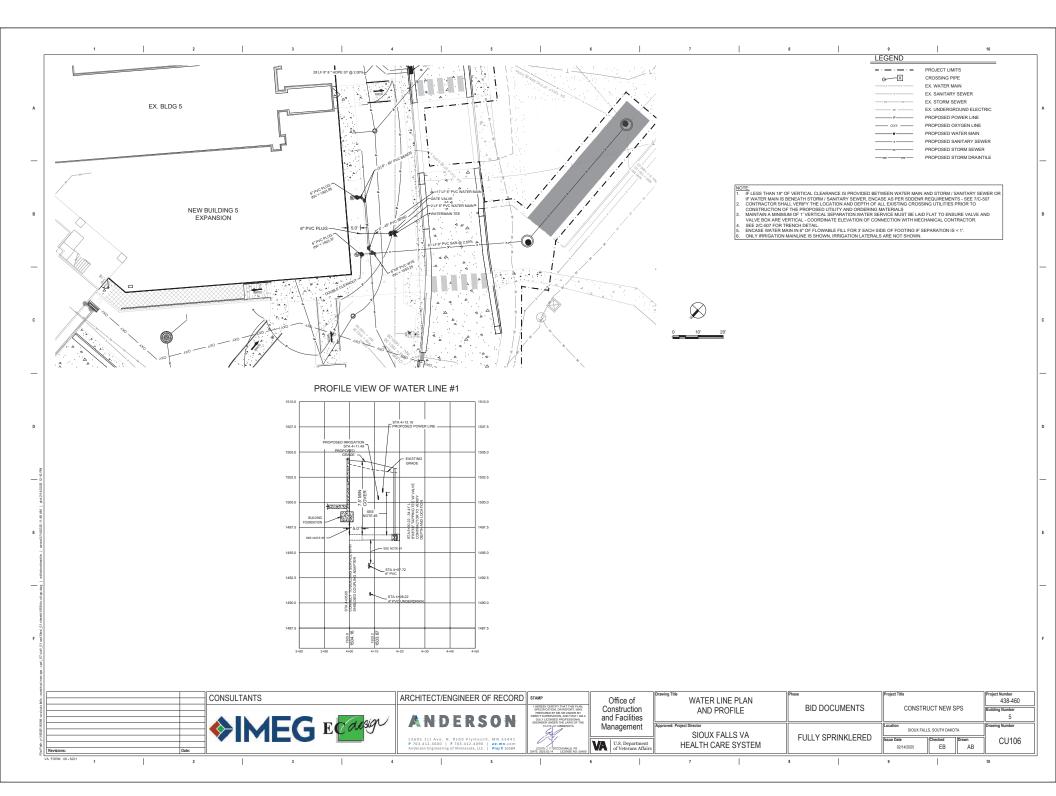


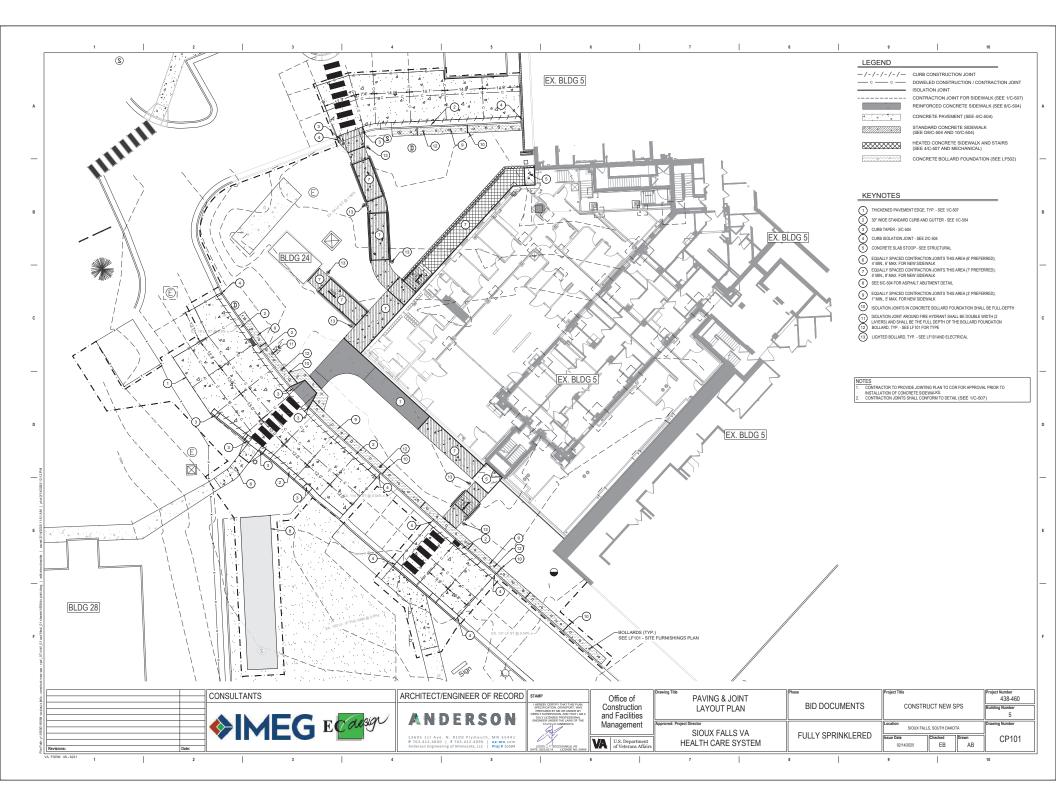


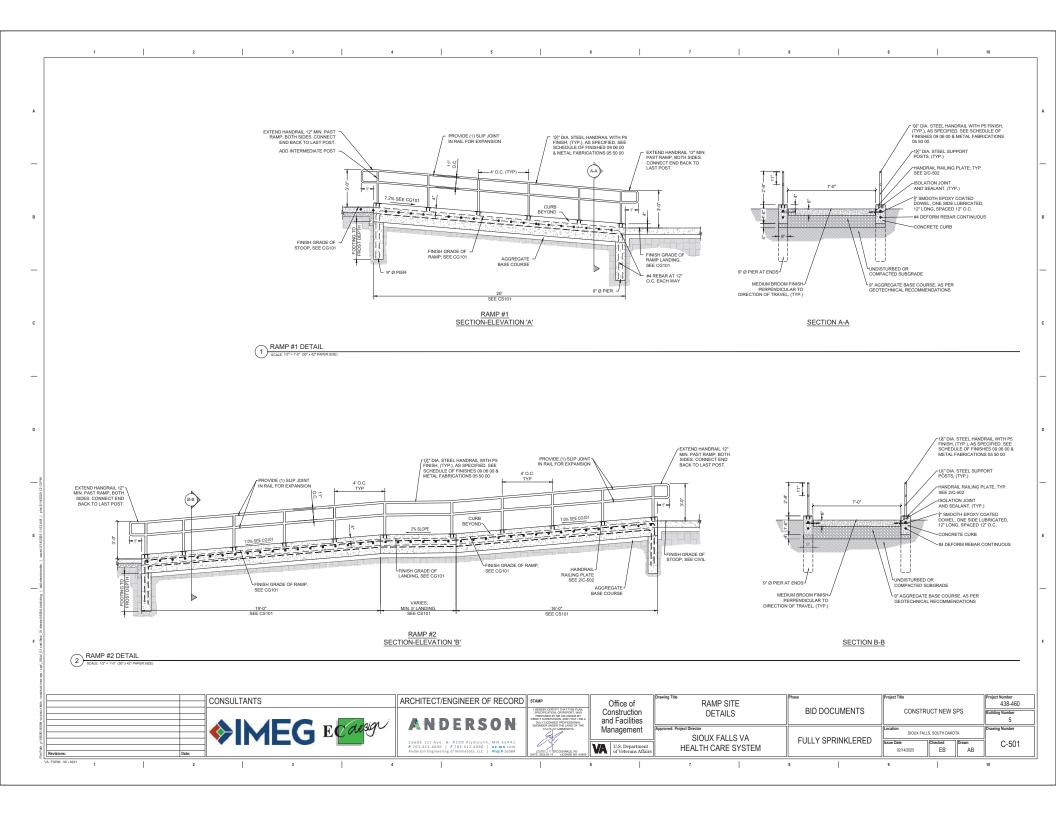


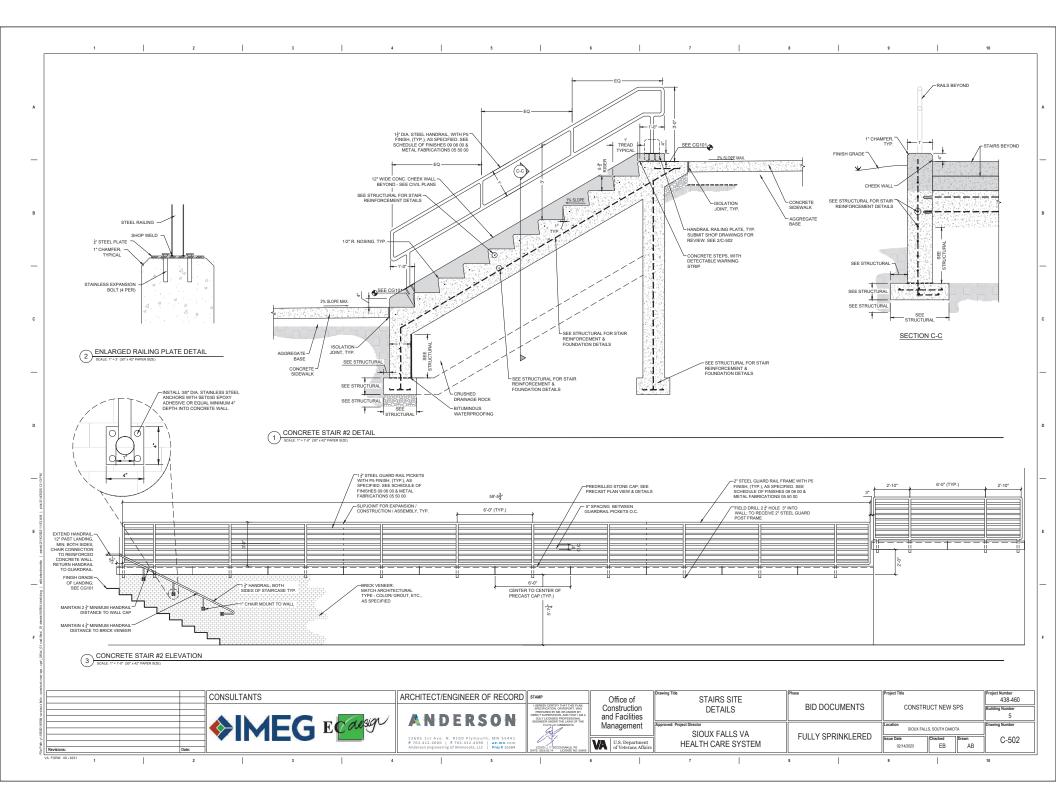


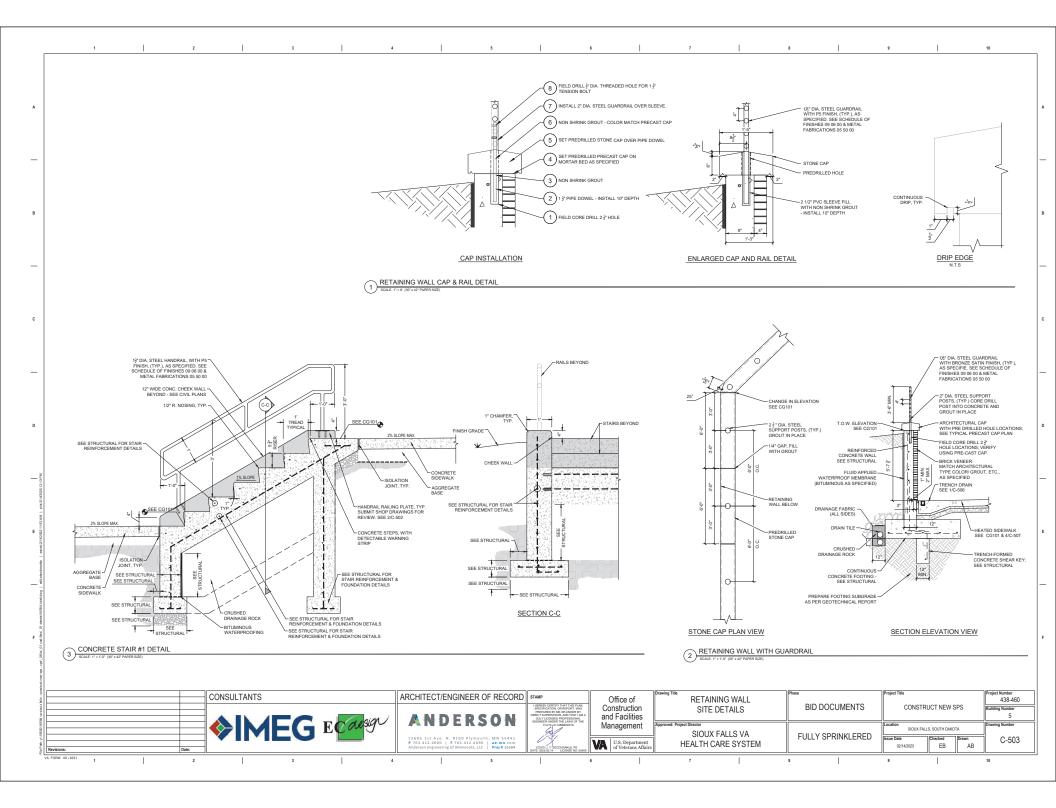


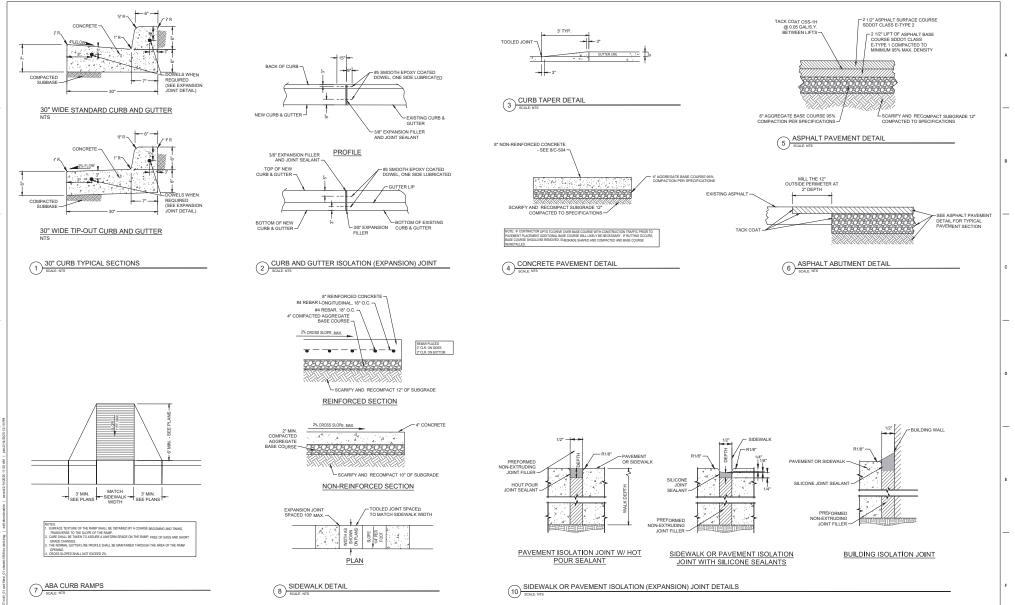










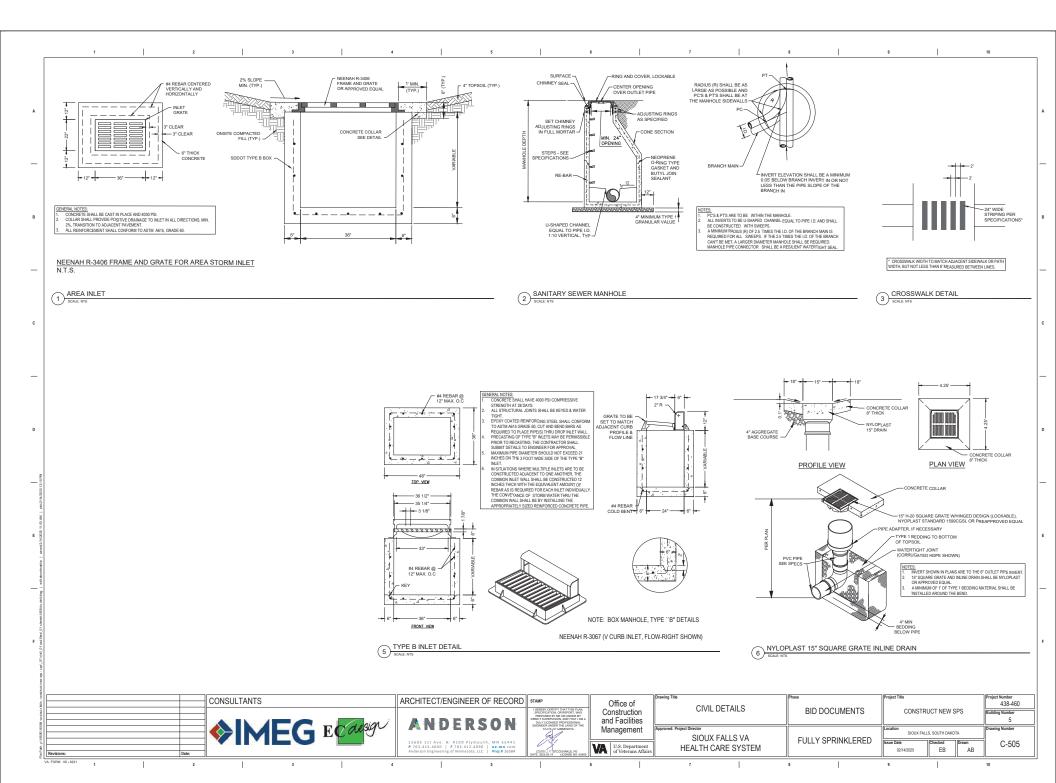


			CONSULTANTS	ARCHITECT/ENGINEER OF RECORD		Office of		Phase	Project Title			Project Number 438-460
				ANDERSON	I HEREBY CERTIFY THAT THIS PLAN, SPECIFICATION, OR REPORT, WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION, AND THAT I AM A DULY LICENSED PROFESSIONAL	Construction and Facilities	CIVIL DETAILS	BID DOCUMENTS	CONSTR	RUCT NEW S	PS	Building Number 5
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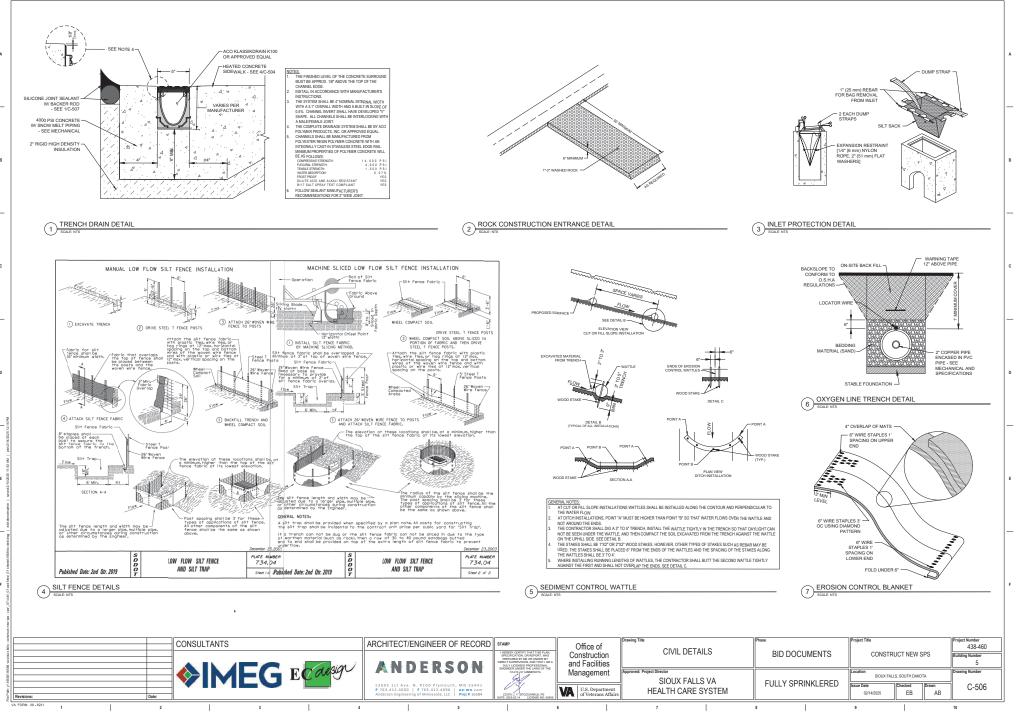
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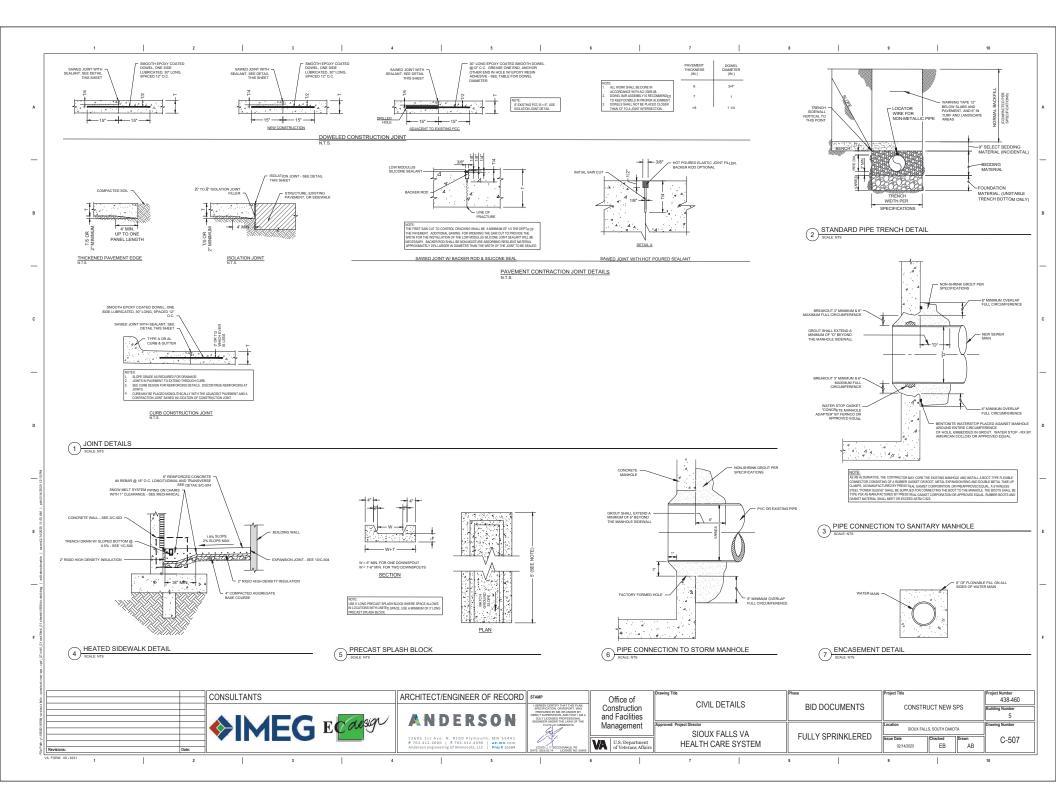
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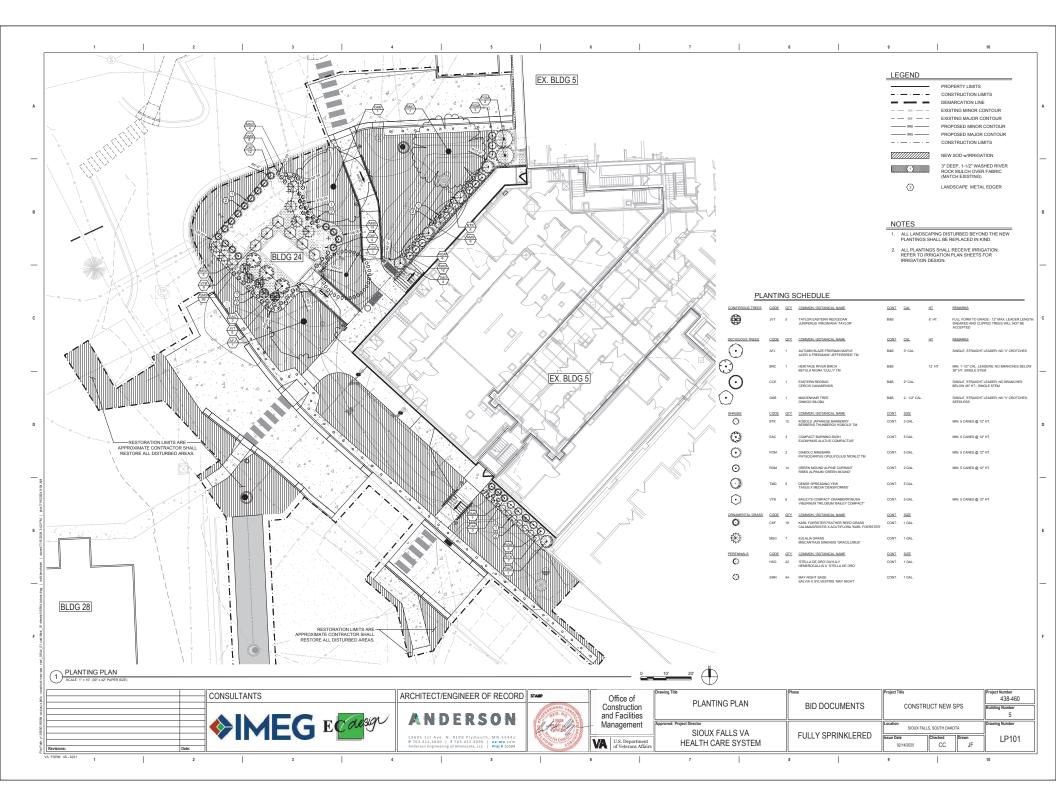
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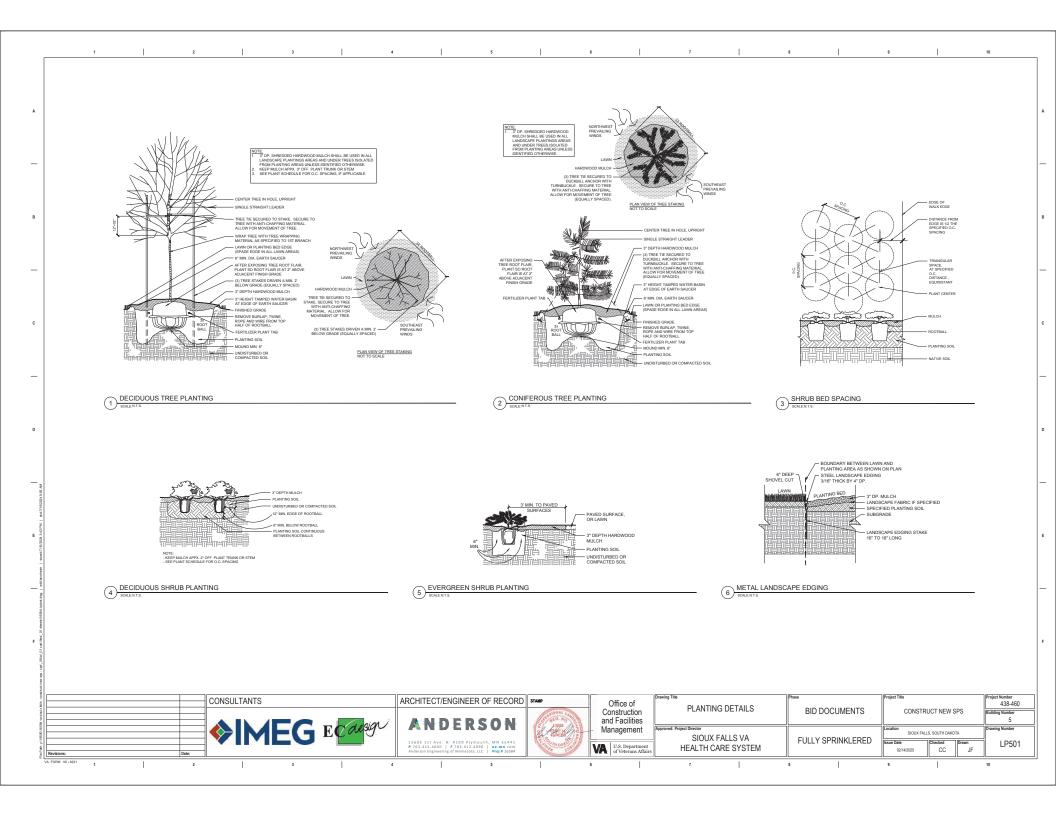


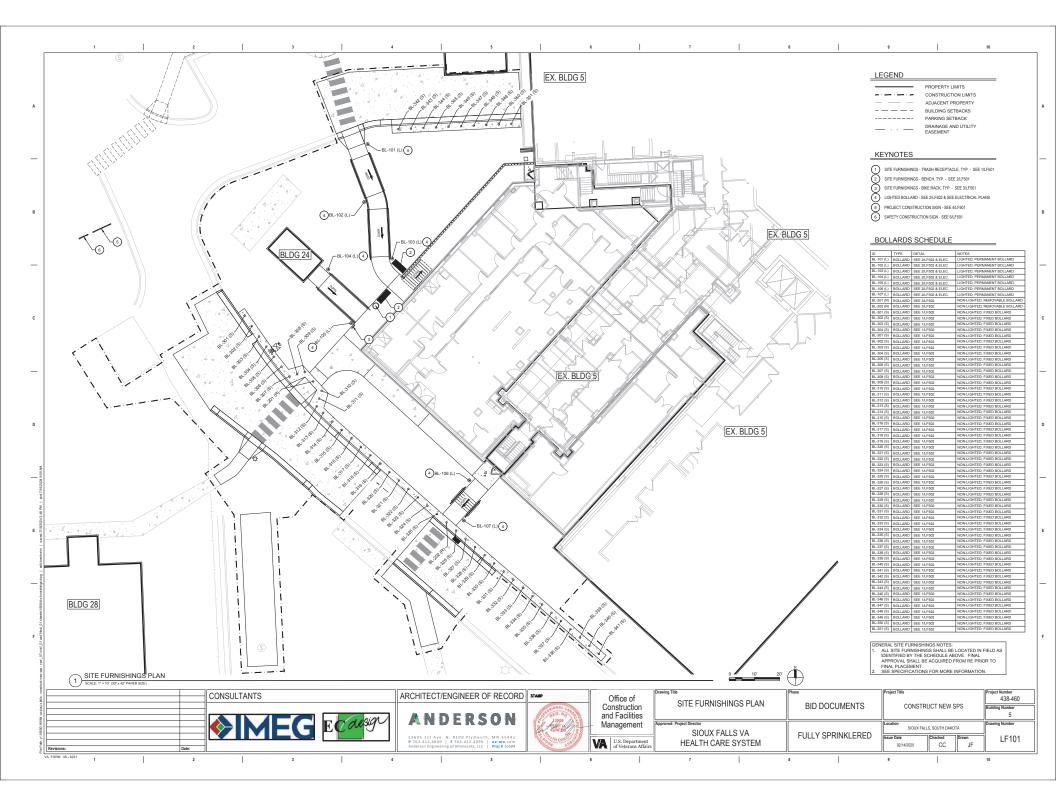


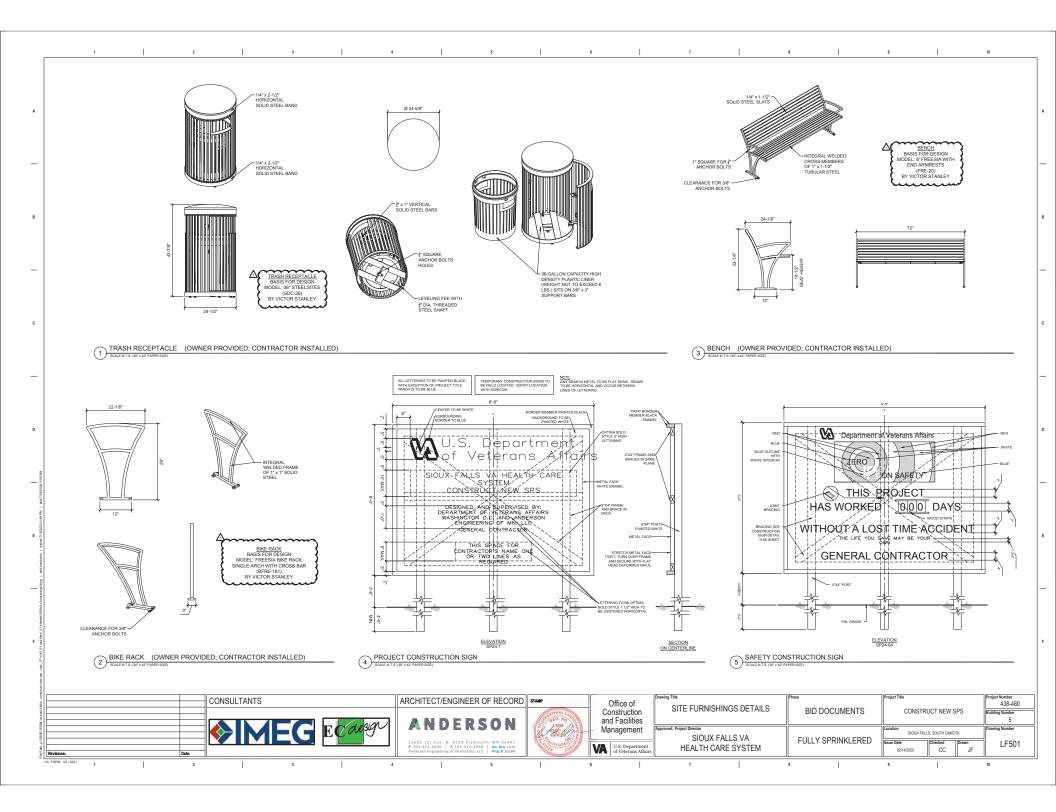


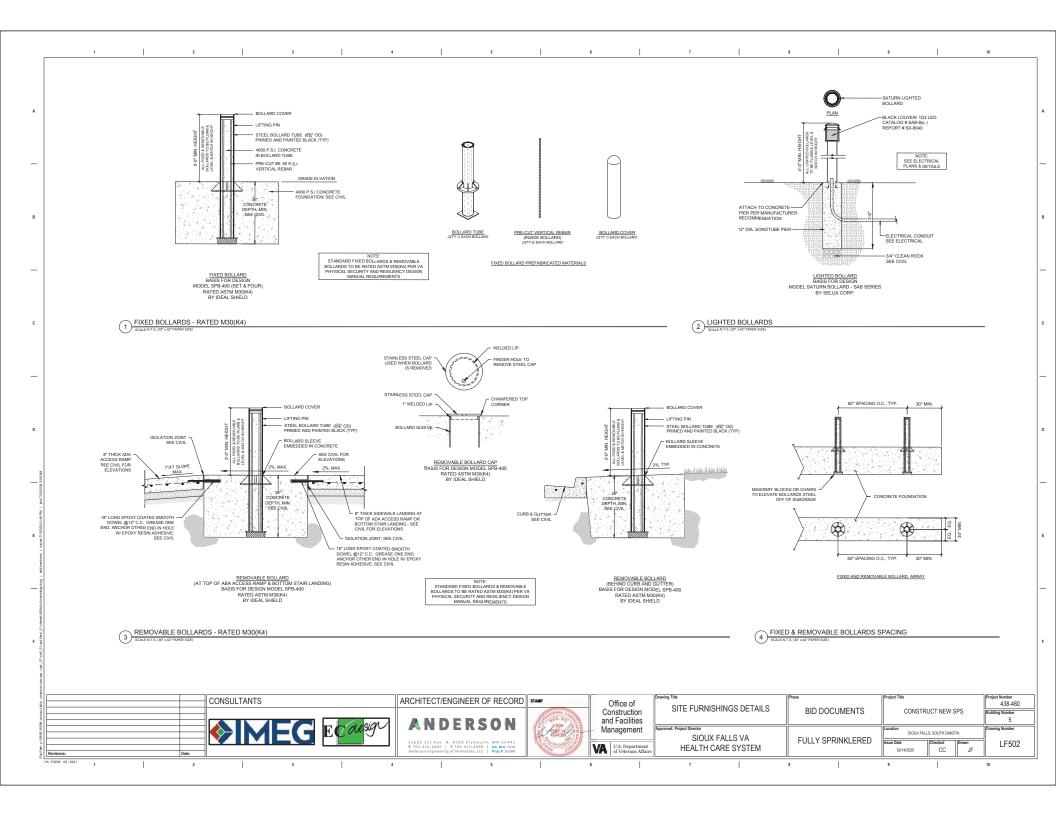


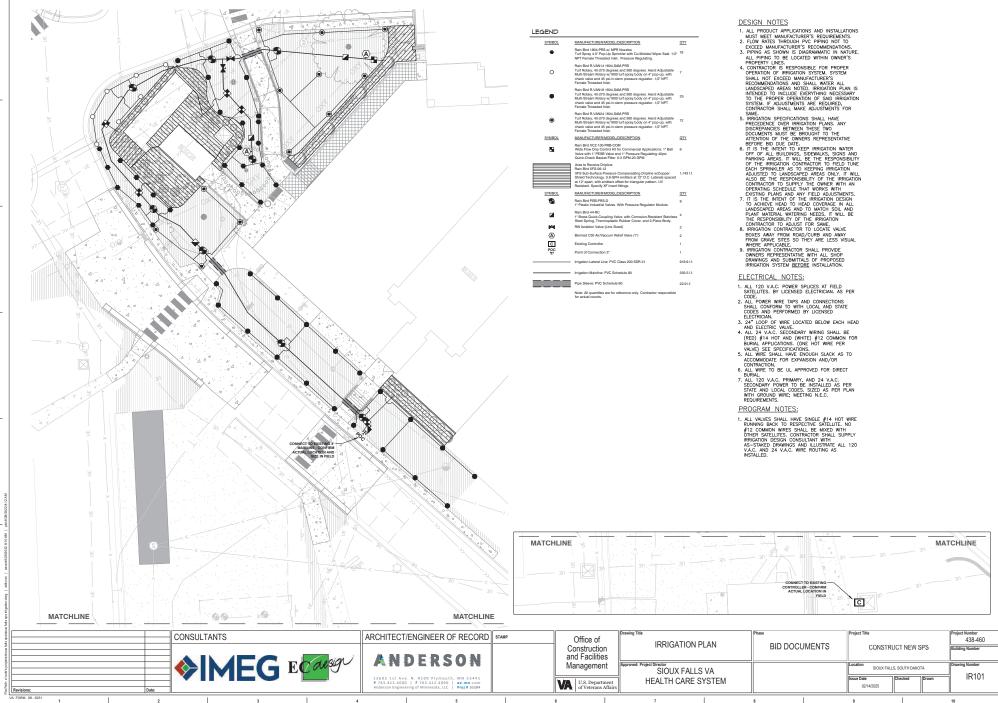


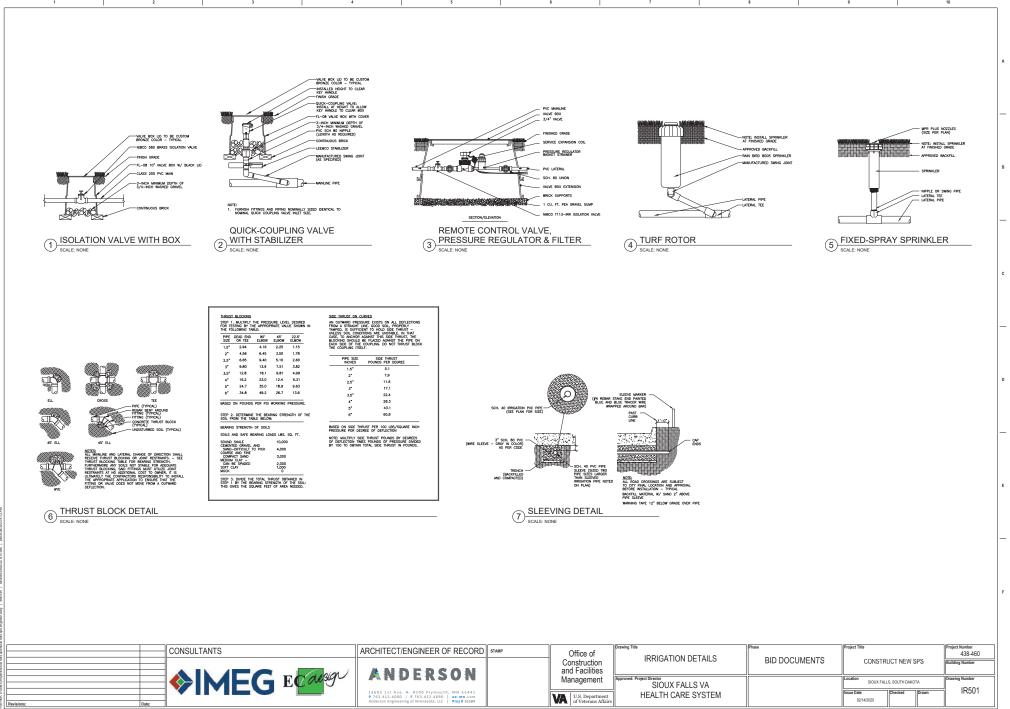


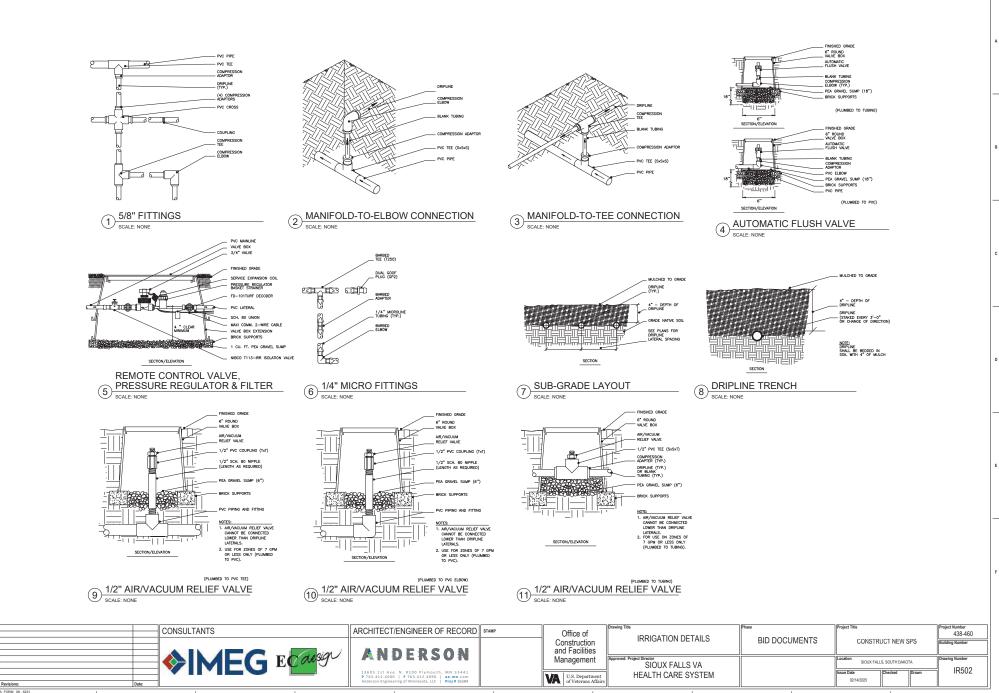












AUTOMATIC FLUSH VALVE PLUMBED TO TUBING (TYP.) ₩œ**.** BACKFLOW PREVENTER ___ BACKFLOW PREVENTER _ POINT OF CONNECTION PVC FLUSH MANIFOLD (rv)-#00-REMOTE CONTROL VALVE WITH FILTER AND PRESSURE REGULATOR REMOTE CONTROL VALVE WITH FILTER AND PRESSURE REGULATOR MANIFOLD-TO-ELBOW CONNECTION (TYP.) ΠŬÆT POINT OF CONNECTION PVC SUPPLY MANIFOLD BACKFLOW PREVENTER AIR/VACUUM RELIEF LATERAL BLANK TUBING CENTERED ON MOUND OR BERM (TYP.) MANIFOLD -TO-ELBOW CONNECTION - PVC SUPPLY MANIFOLD AIR/VACUUM RELIEF VALVE PLUMBED TO BLANK TUBING AT EACH HIGH POINT (TYP.) AIR/VACUUM RELIEF VALVE PLUMBED TO BLANK TUBING AT EACH HIGH POINT MANIFOLD-TO-TEE CONNECTION REMOTE CONTROL VALVE WITH FILTER AND PRESSURE REGULATOR AIR/RELIEF LATERAL BLANK TUBING CENTERED ON MOUND OR BERM PERIMETER LATERAL 2" TO 4" FROM EDGE DRIPLINE LATERAL AREA PERIMETER AUTOMATIC FLUSH VALVE - PLUMBED TO TUBING (TYP.) AIR/VACUUM RELIEF LATERAL BLANK TUBING CENTERED ON MOUND OR BERM AREA PERIMETER AIR/VACUUM RELIEF VALVE DRIPLINE PLAN PVC FLUSH MANIFOLD AIR/VACUUM RELIEF VALVE PLUMBED TO BLANK TUBING AT EACH HIGH POINT FINISHED GRADE PERIMETER LATERALS 2" TO 4" FROM EDGE - Dripline Blank - Tubing AUTOMATIC FLUSH VALVE - PLUMBED TO TUBING BLANK BLANK BLANK BLANK BLANK BLANK BLANK BLANK BLANK PERIMETER LATERALS 2" TO 4" FROM EDGE SECTION Ŏ \odot PLAN PLAN 1 MOUND LAYOUT 2 END FEED LAYOUT SCALE: NONE REMOTE CONTROL VALVE WITH FILTER BACKFLOW PREVENTER AUTOMATIC FLUSH VALVE PLUMBED TO TURING L∞# AUTOMATIC FLUSH VALVE PLUMBED - TO TUBING HIGH POINT ON SLOPE REMOTE CONTROL VALVE WITH FILTER AND PRESSURE REGULATOR POINT OF CONNECTION PVC SUPPLY MANIFOLD AREA PERIMETER MANIFOLD-TO-ELBOW CONNECTION (TYP.) BACKELOW DREVENTER _ PVC TEE (TYP). _ POINT OF CONNECTION PVC FLUSH MANIFOLD COMPRESSION TEE (TYP.) POINT OF CONNECTION DRIPLINE AIR/VACUUM RELIEF VALVE - PLUMBED TO PVC Ó BACKFLOW PREVENTER COMPRESSION ELBOW (TYP.) COMPRESSION TEE USE WHEN SLOPE EXCEEDS 3% IN DIRECTION OF FLOW IN-LINE, SPRING CHECK VALVE TO HELP CONTROL LOW-HEAD DRAINAGE (TYP.) PVC FLUSH MANIFOLD START CONNECTION: INSTALL PRESSURE REGULATOR AT EACH MANIFOLD CONNECTION SERVICING DRIPLINE SYSTEM (TYP.) AREA PERIMETER REMOTE CONTROL VALVE WITH FILTER AND PRESSURE REGULATOR PERIMETER LATERALS 2" TO 4" FROM EDGE - PVC SUPPLY MANIFOLD __ POINT OF CONNECTION 鹵 MANIFOLD-TO-ELBOW CONNECTION (TYP.) PERIMETER LATERALS - PVC ELL (TYP.) BACKFLOW PREVENTER ÷ _ ISLAND PERIMETER (TYP.) PVC FLUSH MANIFOLD REMOTE CONTROL VALVE WITH FILTER AND - PRESSURE REGULATOR COMPRESSION TEE Ó DRIPLINE PRESSURE-COMPENSATING DRIPLINE - LATERAL AIR/VACUUM RELIEF VALVE PLUMBED TO DRIPLINE AT EACH HIGH POINT (TYP-) PVC SUPPLY MANIFOLD MANIFOLD-TO-ELBOW CONNECTION AR/VACUUM RELIE VALVE AUTOMATIC FLUSH VALVE - PLUMBED TO TUBING (TYP.) PVC SUPPLY MANIFOLD 1 100 PERIMETER LATERALS 2"TO 4" FROM EDGE AR/VACUUM RELIEF VALVE PLAN AUTOMATIC FLUSH VALVE PLUMBED TO TUBING (TYP.) PLAN Ó PLAN PLAN 7 ISLAND LAYOUT (TRIANCI II AR) (ODD CURVES) (PRESSURE-COMPENSATING DRIPLINE) IRREGULAR LAYOUT SLOPE LAYOUT (5) SCALE: NONE

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_ POINT OF CONNECTION

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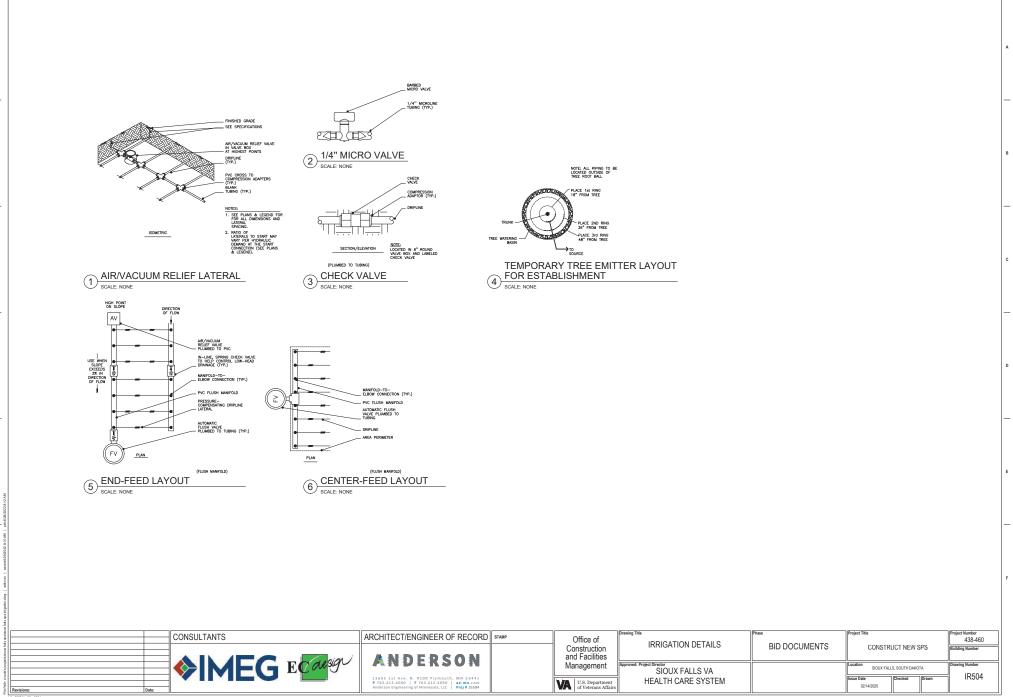
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Case

Other Bars

TENSION LAP SPLICE LENGTH

Case 2

Notes: 1. Tables are for normal weight concrete with Grade 60 uncoated reinforcing bars. For lightweight aggregate, values in the table by 1.33.

A = struct said = 12-0 Coordinate control joint layout with floor finish requirements.
 Control joint depth to be 1^{*}, using an early entry saw.
 Cut control joints with an early entry saw is soon as possible without damage to the slab surface.

Provide 6x6-W1.4xW1.4.W.W.F. in all slabs on grade, unless noted otherwise. All mesh to be lapped a mi Provide prefabricated sheets in lieu of rolled mesh. Reinforce with (2)-#5 x 3-0" long at all re-entrant (inside

4. Coordinate all floor finishes, slopes, recesses, floor drains, gutters, etc. with all disciplines (arch., mech., etc.).

6. Do not provide control joints in structural slabs, slabs on metal deck, or precast topping, unless noted off

Provide thickened slabs at masonry partitions and construction joints as detailed below.

Partitions, see arch. plan for size & location

Provide a preformed isolation joint in concrete slab at columns. The isolation joint can be either a circular or diamond shaped pattern.

3. Place slab reinforcing between 1/4 and 1/3 of slab thickness down from top of slab.

Compression lap splices (only where indicated on drawings) for Grade 60 uncoated reinforcing bars shall be 30 times the bar diameter.

Beams and columns: Case 1: Concrete ocurr at least 1.0 times the bar diameter and center-to-center spacing of at least 2.0 times th bar diameter. Case 2: Concrete cover least than 1.0 times or center-to-center spacing less than 2.0 times the bar diameter. Urber members:

All other members: Case 1: Concrete cover at least 1.0 times the bar diameter and center-to-center spacing at least 3.0 times the bar diameter.

bar diameter. Case 2: Concrete cover less than 1.0 times the bar diameter or center-to-center spacing less than 3.0 times the bar diameter.

joints at column centerlines and at the following maximum

U.

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CONSTRUCTION JOINT (STOP MESH AT CONSTRUCTION JOINT)

(Partition width) +6" or minimum of 1'-0"

2. Top bars are horizontal bars with more than 12" of concrete cast below the bars.

Bar Size

4. Cases 1 and 2 are defined as follows:

CONCRETE SLAB AND JOINT NOTES AND DETAILS:

Control Joints (C.J.) - Locate saw cut contro approximately square panels

1.0

+4

PARTITION FOOTING (MESH CONTINUOUS)

TYPICAL LINTEL TYPES AND NOTES:

a. Steel angle lintels:

provimately square panels Concrete slabs on grade: i. 4*-5* thick slab = 12-0*

STEEL DECK NOTES:

GENERAL CONSTRUCTION NOTES:

- 1. Reference Standards: Unless noted wise, all standards shall be current edition, with latest addenda. If applicable Contractor shall verify all existing dimensions, member sizes, and field conditions prior to any demolition, flabrication, construction, or installation and notify Btructural Engineer of Record II conditions, materials, sizes, and dimensions are different flow flows shown.
- The contract structural drawing not indicate the means or method of construction. The contractor is solely resp during all phases of demolition, construction, and installation.
- The finished structure has been designed for the loading indicated below. It is the responsibility of the contract their speciality Engineeity to review and use means and methods to adequately address loading on the struc-construction including, but not imited to, wind, survey, sessinic, underprinting, material stronge, and equipment.
- Cross reference all dimensions and details with architectural and mechanical drawings before commencing any fabrication and/or construction.
- Details and conditions not specifically shown shall be constructed in accordance with details shown for similar or and materials.
- Shop drawings prepared by suppliers, sub-contractors, etc. shall be reviewed, coordinated, and signedistamped contractor prior to submitting to the Structural Engineer of Record. The Structural Engineer of Record's review of drawings, product data, design calculations, etc., does not releve the contractor from complying with the contract drawings.
- Verify location of all box outs and openings. Opening sizes and locations shown for pipes, ducts, mechanical units, etc are for general information only and shall be verified with all trades before commencing the work. 9. Contractor is solely responsible for protection of the existing building during all phases of construction.
- No structural repairs, corrections, or alterations of work affecting a structural member shall be made without the ap of the Structural Engineer of Record, Design and/or review may be an additional service.
- 11. Do not scale the drawings.

DESIGN CRITERIA LOADS AND STRESSES

- 1. International Building Code (2018) ^o Minimum Desion Loads for Buildings and Other Structures (ASCE 7-16).

DESIGN LOADS-

Ss = 0.090 g S₁ = 0.035 g Mapped Spectral Response Acceleration Parameters, S₁ and S₁ Site Class Design Spectral Response neters, Sox & Sor n = 0.056 g Seismic Design Category Pasir Seismic Force-Resisting System "A" Steel ordinary concentrical brace frame and steel ordinary Seismic Response Coefficients Design Base Shear Analysis Procedure Used Cs = 0.01 V = C₃ * W Equivalent Lateral Ultimate Design Wind Speed (3-sec gust), V,

Nominal Design Wind Speed (3-sec gust), Vau	93 MPH
Risk Category	IV
Wind Exposure	°C"
Internal Pressure Coefficients	GCµ = +/- 0.18
	SNOW LOAD DATA*
Ground Snow Load, P.	40.PSF
Snow Exposure Factor, C+	1.0
Snow Load Importance Factor, I	12
Thermal Factor, C.	

Slope Factor, 0 Flat Roof Snov C₄ = 1.0 Heated P₄ = 35 PSF + drifting and R.

FLOOR LIVE LOADS

150 PSF Mechanical/Electrical areas 100 PSF Stairs, and exits

CONCRETE: (I'c) at 28 Days

3000 PSI Footings 3500 PSI Stab on grade Imax w/c = 0.45. fly ash not p

3500 Poi balo on globa (max wic = 0.44, my san not permitted, no entrainin 4000 Poil Balo on steel deek, topping salos (max wic = 0.45, my san not ext 4500 Poil Pers, foundation walls, and exterior salas [5%-7% air content] 4500 Poil Retaining walls, basement walls, pile caps, and grade beams 7000 Poil Non-shrink group balo baseplates

All exterior concrete work shall have 5% to 7% air entr

STEEL: (Fy)

60,000 PSI ASTM AS15 grade 60 minforcing 60,000 PSI ASTM A706 weldable neinforcing 50,000 PSI ASTM A706 weldable neinforcing 50,000 PSI ASTM A36 plates, Charnels, and angles, di 50,000 PSI ASTM A36 plates, Charnels, and angles, di 50,000 PSI ASTM A306 plates C structural plates (HSS)

92,000 PSI ASTM A325 h 36,000 PSI ASTM F1554 50,000 PSI ASTM A108 h

FOUNDATION LOADS:

2,000 PSF soil bearing, based on soil report prepared by Geotek Engineering & Testing Services, Inc., dated April 17, 2019, (report # 19-225). See Geotek report for required site preparation. Steel helical piles and micropiles shall be designed for unfactored load indicated on sheet SB102.

LATERAL EARTH PRESSURE:

35 PCF Active Lateral Earth Pressur STEEL HELICAL PILES AND MICROPILES:

- Piles shall be installed within the following tolers Top Elevation +1-1/2" Plamb within 1" Pile Centerline within 3"

Revisions:

/A FORM 08-623

Piles shall be designed to provide the required load capacities shown on plans with a 1/2^e maximum at any pile and a 1/4^e maximum vertical differential settlement between any two adjacent piles uon.

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Internet instructor, pills to the greates enterly practicalite, except where otherwise approved by the Owner or A A load test shall be deemed acceptable provided the maximum test load is applied without pills failure and the deflection of the pile head at the design load is lises than 0.25° on on the drawings. Failure is defined when continuous jacking is required to maintain the load.

Architect shall be notified immediately if obstructions are enco

The testing and inspection agency and installing contractor shall keep a record or log of each pile as installed sho location, top and bottom elevations, diameters, date installed, type of strata encountered, and any other pertinent

6. The pile contractor shall cubmit calculations for the design of piles. Those calculations shall be signed and sealed by an engineer licensed in the project's jurisdiction.

Pile contractor shall submit shop drawings for review showing dimensioned pile layout plan and installation sequence, pile components and details, corrosion protection system, and pile top atachment.

TEMPORARY BRACING

- Provide temporary lateral support for all walls where grade varies on the two sides until slab has reached its design strength. 2. Provide required temporary bracing for structural steel until permanent bracing and walls are in place.
- Provide temporary bracing for all walls, concrete, masonry, light gage metal, or wood until they are of adequate design strendth and are properly anchored in final form.
- Provide temporary shoring for all existing walls, floors, and roof members until new construction is in place and properly anchored or cured in final form.
- All temporary shoring is to be designed by a specialty shoring contractor, by a Professional Engin of the project, at the expense of the contractor.
- 6. Shore all foundation walls as required before backfilling and compacting.
- 7. Contractor shall provide adequate bracing and shoring during all phases of const

GENERAL FOUNDATION NOTES:

- All foundation excavations, backfill, and compaction shall be inspected and certified by a qualified solis testing firm prior to the construction of any footings. All reports are to be submitted to Structural Engineer of Record in a timely manner. Cross reference all architectural, mechanical, electrical, and structural drawings to assure proper dimensions and placement of all anchor rods, inserts, etr.
- 3. All footing elevations are shown to top of footings, unless noted otherwise.
- 4 All footions are centered under walls or columns above unless roted otherwise
 - 5. Continuous wall footings up through 1'-8' wide to be 10' thick. Footings over 1'-8' wide to be 12' thick, unless noted

 - Provide wall footing reinforcement as follows: Footings up through 2-0" wide = (2,45 cont. Footings 2-1" through 3-0" wide = (3,45 cont. & #5 (§) 12" transv. Footings 3-1" through 3-4" wide = (3,45 cont. & #5 (§) 12" transv. See details for minforming in all factions over X-5" wide
 - Provide 90 degree bend in all footing dowels. Cast dowels in footings for columns, piers, and wals above. Dowels to be the same number and size as the vertical ineffocing, unless noted otherwise. See General Concrete Notes or General Macomy Notes for required lag rength. 8. Rebar and anchor rods to be securely tied in place prior to placing concrete (i.e. no "wet-sticking" is allowed

- GENERAL CONCRETE NOTES:
- Concrete construction shall comply with the provisions of the "Building Code Requirements for Structural Concrete," ACI 318-14. 2. The "ACI Detailing Manual" shall govern detailing and fabrication of all reinforcing steel, unless noted otherwise.
- Reinforcing steel supplier to provide all accessories, chairs, spacing bars, and supports necessary to secure steel in accordance with "Manual of Standard Practice" by the Concrete Reinforcing Steel Institute. Clay brick is not allowed.
- 4. Provide minimum clear concrete cover for all reinforcement as follows: Cast against and permanently exposed to earth = 3"
- Exposed to earth or weather: #5 bars and smaller = 1 1/2" #6 bars and larger = 2"
- Not exposed to weather or in contact with ground: Slabs, walls, & joists (#3 to #11 bars) = 3/4" Reaves, ciridess and columns, primary reinforcement, lies, stimups, or spirals = 1 1/2"
- Provide comer bars at all comers and intersections of walls, grade beams, and edge beams. Comer bar to be the same size and spacing as all horizontal bars. At openings in structural slabs or walls, provide a minimum of (2)-86 bars each side of opening. Bars are to extend a minimum of 3-0° beyond contents of openings, unless noted otherwise. Provide (1)-45 x 4-0° long diagonal bar at each corner of opening in each face of wall of sub.
- Provide minimum concrete wall reinforcing as follows: (unless noted other
- 6" & 6" concrete walls: #4 @ 16" o.c. vert & #4 @ 10" o.c. horiz (center in wall)
- 10° concrete walls: #4 @ 16° o.c. vert & #4 @ 16° o.c. horiz (each face)
- 12" concrete walls: #4 @ 16" o.c. vert & #4 @ 12" o.c. horiz (each face)

4000 PSI:

Bar Size

CONSULTANT

Date:

16" concrete walls: #4 @ 16" o.c. vert & #4 @ 12" o.c. horiz (each face)

noted otherwise. Install anchors in strict conformance with anchor manufacturer's instructions. Anch not be made without written permission from the Structural Engineer of Record.

- Provide vertical control joints in exposed concrete walls at a maximum of 30'-0" intervals. See typical details for Control Joint and Construction Joint Detail.
- 9. No aluminum of any type shall be allowed in the concrete work, unless coated to prevent reaction with concrete Maximum outside dameter of embedded conduit shall be no larger than 1/3 of the slab hickness. This restriction applies to the total height at conduit crossings. The conduit shall be placed such that it does not significantly impair the strength of monthmicity.

No pipe or conduit of any type shall be placed in structural concrete members without written approval from the Structural Engineer of Record.

13. Composite slabs and beams are designed to support the dead load of the wet concrete plus normal construction loads without requiring temporary shoring. Some deflection of the deck and beams will occur when the wet concrete is place. The contractor shall include in the bid the cost of the additional concrete quantity caused by the deflection of the beams and deck.

14. Do not weld rebar, unless Weldable Rebar is provided and its use is approved by the Structural Engineer of Record

Lap splice lengths in continuous reinforcing shall be tension lap splices and are shown below, unless noted otherwise on drawings or details:

CLASS BITENSION LAP SPLICE LENGTH

Other Bars

Other Ba

4

GENERAL STEEL NOTES: Posi-installed anchors in concrete shall be ICC approved for use in cracked concrete. Approved anchors shall be HIII: Kwk Bolt TZ Expansion Anchors (ESR-1917) or a HIII HIT-HY 200 Adhesive Anchorino System (ESR-3187), unless

Construction of structural steel shall comply with the provisions of "AISC 360-16 Specification for Structural Steel Buildings" and "AISC 341-16 Seismic Provisions for Structural Steel Buildings."

1. Verify size and location of all mechanical, U.V., U.H., louver, and duct openings with mechanical contractor.

(1) L 3 1/2" x 3 1/2" x 1/4" for each 4" thickness of wall for spans up to 4-0".

(1) L 5" x 3 1/2" x 5/16" (LLV) for each 4" thickness of wall for spans up to 5-0".

(1) L 6" x 3 1/2" x 5/16" (LLV) for each 4" thickness of wall for spans up to 6'-0".

For all openings through masonry walls not shown, including mechanical and electrical openings, provide one of the following: (unless noted otherwise)

All steel lintel beams to bear a minimum of 8" on grouted or solid masonry, unless noted otherwise. All steel lintel angles to bear a minimum of 8" on solid or grouted masonry, unless noted otherwise.

- All shop connections shall be welded or bolted, field connections shall be bolted, unless noted otherwise. Bolted connections shall be Bearing Type (area;-tiptaned) and shall be made with a minimum of 3/4°a ASTM ASS5-N Bolts Direct-Transic Indicators are acceptable substitutions.
- 3 All weirfs as per latest specifications of the AWS F70xx electrories

4. All lintels in exterior walls to be hot-dipped galvanized, unless noted otherwise.

- 4. Before encasing steel columns in concrete or masonry, paint column bases and tops of anchor rods with aschaltic paint The structural fabricator shall furnish all plates and angles cast in bond beams, concrete walls, or columns to support steel joists, beams, and steel deck.
- 6. 'C' denotes beam is continuous over columns. 'S' denotes beam simple shear splice.
- All steel beams shall be true to line and elevation, column base plates grouted, and anchor rods light before any loads are placed.
- 8. All column base and cap plates to be welded around all sides All welds not specified are 3/16" fillet weld, continuous and/or all around.

ARCHITECT/ENGINEER OF RECORD

ANDERSON

13605 1ec Ave. II. #100 Plymouch, MN 99441 F 763.412.4008 | F 769.412.4090 | as-ant.com Anderson Registering of Minnesista, MC | Prij # 16584

- 10. Structural fabricators shall show all welding requirements on structural steel shop drawings.
- Fabricator shall select AISC simple shear connections for composite beams capable of carrying the reaction load indicated or the reaction load calculated and based on tibulary area or at a minimum 75% of the total shear capacity Connection design shall also be check for blact reaction given on shear SG000.
- 12. Cuts, holes, or openings required in structural steel members for the work of other trades shall be shown on the shop drawings. Burning of holes and cuts in structural steel members in the field shall not be allowed, except by written permission from the Structural Engineer of Record.

13. The top of all beams receiving shear studs shall not be painted.

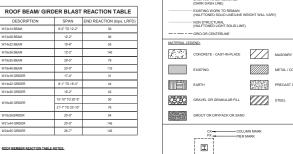
- 4. The contractor shall provide 4000 pounds of structural steel contingency material to be fabricated and erected as directed by the Shuchural Engineer of Record. Cost of material, labor, delivery, and associated services are to be included in the bid amount.

17. Location, type, diameter, length, and spacing of shear stud connectors shall be detailed on the shop drawings.



NEG. Ne 11128 MICHAEL F

19. Shear stud connectors shall be manufactured by Nelson Stud Welding Co. or equal contoming to ASTM A108, and shall be field applied with automatic welding equipment through the composite steel deck with the use of a proper femule. Remove fermines after welding.



STRUCTURAL ABBREVIATION KEY

BOTTOM BETWEEN SETWEEN COLD FORM STEEL FRAMING COMPTENT OF THE TENDON COMPTENT OF THE TENDON COMPTENT OF THE TENDON CONCRETE MASONRY UNIT CONCRETE CONCRETE CONCRETE CONCRETE CONTROLOUS

ABBR: DESCRIPTION: # NUMBER OR POUNDS

DEGREE

(E) A.B. ARCH B.O. bf BM B.N. BOTT BTWN CFSF CGP CLR

CL CMU COL CONC CONST CONST CONST CONST CONT DL DET DWG DWL EA EF EFF EL

ELEC EMBED E.N. EOD EOS EDGE NAILING EDGE OF DECK EDGE OF SLAB EQUAL

EQ EQUIF ETC EW EXP FXT

EXT f'DN FIN FT GALV HORIZ HSB JT K, KIP KSF KSI

LBS LLH LLV LONG. LSV LSV LT WT MAX MECH MIN NIC NTS OC OPNG OSB PCF.

PL PLF PSF PSI PT R REINF REQD RTU

SCHED SCHED SFRS SM SL SPECS SQ STIFF STL SYM T&B T.O. TC TEMP

SHEET INDEX - STRUCTURAL

SHEET TITLE

DIAMETER EXISTING ANCHOR BOLT ARCHTECT, JURE, JURAL BOTTOM OF BEAM FLANGE WIDTH BRACE FRAME BEAM BOTTOM BETWEEN COLD EDM STEEL ERAM

CONTINUOUS COORDINATION DIAMETER DEAD LOAD DETAIL

DRAWING DOWEL EACH EACH FACE EFFECTIVE

EXPANSION EXTERIOR CONCRETE COM FOUNDATION FIELD NAILING

FOOT FOOTING YIELD STRESS GAGE OR GAUGE GALVANIZED

HORIZONTAL HEADED STUD ANCHOR HIGH STRENGTH BOLT

HIGH STRENGTH BULL JOINT KLOPOUND (1.000 POUNDS) KIPS PER SQUARE FOOT KIPS PER SQUARE INCH LENGTH POUNDS LIVE LOAD LIVE LOAD LIVE LOAD

LONG LEG HORIZONTAL LONG LEG VERTICAL

LONG SIDE HORIZONTAL LONG SIDE VERTICAL

OPPOSITE HAND OPENING ORIENTED STRAND BOARD POUNDS PER CUBIC FOOT

POUNDS PER SQUARE INCH POST-TENSION, -ED, -ING

STRET-LONG SPRCENTLORG SPRCENTLORG SPRCENTLORG SPRCENTLORG SUPPORT SUPPORT STREL SYMMETRICAL SYMMETRICAL SYMMETRICAL SYMMETRICAL SYMMETRICAL STREL SYMMETRICAL STREL SYMMETRICAL STREL STR

TYPICAL UNLESS OTHERWISE NOTED VERIFY IN FIELD WITH WORK POINT WEIGHT

oject Numbe

438-460

Building Number

SG000

5

IEM EAM FLANGE THICKNES THE HICK THANS TRANSVERSE TYP TYPICAL UON UNLESS OTHERWISE NO' VERT VERTICAL VIET VERTICAL WERT WERTCAL WW WTH WP WORK POINT WT WEGHT WW WELDED WIRE REINFORD

ROOF TOP UNIT

ST-TENSION, -ED, -ING DIUS NFORCING, -MENT, -ED DUIRED

SLIP CRITICAL SCHEDULE SEISMIC FORCE-RESISTING SYSTEM SMULAR SNOW LOAD SHEET METAL SCREW

POUNDS PER CUBIC FOOT POUNDS PER CUBIC FOOT PENTHOUSE PARTIAL JOINT PENETRATION WELD PLATE POUNDS PER LINEAR FOOT

E IDS PER LINEAR FOOT IDS PER SQUARE FOO

LONG SILLE VERVICE-LIGHTWEIGHT MAXIMUM MECHANICAL MANUFACTURER MINIMUM NOT IN CONTRACT NOT TO SCALE ON CENTER OPPOSITE HAND ORDENING

VIEW KEY

PLAN OR DETAIL NO VIEW NAME

NEW WORK (DARK SOLID LINE/LINE WEIGHT WILL VARY)

EXISTING WORK TO REMAIN (HALFTONED SOLID LINE/LINE WEIGHT WILL VARY)

- NON STRUCTURAL (HALFTONED LIGHT SOLID LINE)

CX

DESCRIPTIO

TEEL DECK (DIRECTION

[##] HEADED STUD ANCHORS (HSA) ON BEAM

BID DOCUMENTS

FULLY SPRINKLERED

STRUCTURAL SYMBOL LIST

Ξ

GENERAL SYMBOLS:

FOUNDATION SYMBOLS:

(100'-0") TOP OF ELEVATION

STEP IN FOOTING

SYMBOL

SYMBOL

STEEL SYMBOLS:

BEAM SPLICE

MOMENT CONN

BRACE ABOVE

BRACE BELOW

SYMBOL

SF#(X'-X'')

----- EXISTING TO BE REMOVED (DARK DASH LINE)

NAME - LEVEL NAME

LINE TYPE KEY:

ADDITIONAL INFORMATION ABS WORK REQUIRED, SPECIFIC TO SHEET AND/OR DETAIL

METAL / COLD-FORM STUD

DETAIL REFERENCE

N/A

DETAIL REFERENCE

DETAIL REFERENCE

SEE PLAN NOTES

3/SF220

SEE FRAME ELEVATION

SEE FRAME ELEVATION

SHEET NO.

NERAL NOTES

ROOF FRAMING PLA

Sioux Falls, SD

leeue Date

02/14/2025

PARTIAL BASEMENT FOUNDATION PLAN

CONSTRUCT NEW SPS

Checked Deaver

MPM/TGL MAQ

GROUND LEVEL COLUMN LOADS ENLARGED PLANS AND SECTION

1/SB200

PRECAST CONCRETE

FOOTING MARK (TOP ELEVATION

INDICATES DIRECTION OF TRUE NORT

PLAN OR DETAIL NUMBER

INDICATES SIMILAR DETAIL REFERENCED

DETAIL REFERRED TO BY SECTION CUT S300 SHEET DETAIL IS LOCATED ON

PLAN OR DETAIL NAME

1/8" = 1'-0" PLAN OR DETAIL SCALE

CONNECTIONS MUST BE DESIGNED FOR THE REACTION LOADS LISTED IN THE TABLES AT A MINIMUM. THESE SHOULD NOT BE COMBINED WITH OTHER LOAD (I.E., BLAST DESIGN COMBO=1.0 * BLAST)

All steel decking shall comply with the specifications of the Steel Deck Institute (SDI). Thickness, type, and pr decks shall be as shown on the drawings.

Field weld 1 12^o sitel roof deck to supporting members with 58^o puddle welds at 38^{id} pattern. Where areas of warped nink nccur. field weld steel deck maximum 6^o o.c. at all supports. Typical, unless noted otherwise.

11/2" steel roof deck shall have; (1)-#10 TEK screw side lap connector installed between adjacent supports (unless noted otherwise).

Composite state dark with concrete sisted shall be welded to all supporting members with SIT's puddle welds at 364
 patient. Fir of each units with space aproach than 5-7; sisted and a primiter edges of units between span supports shall
 all 501 and first screams. 37 are using a set of the first scream primiter.
 all 501 and first screams. 31 are using the scream primiter.
 b) Oting or batters punch
 b) Oting or batters
 b) Oting

See plans and details for composite deck thickness, depth, and profile. All composite steel deck to be galvanized with G-60 coating.

Steel conform deck shall be attached at all supports sufficiently to prevent movement. Steel deck fasteners are not required for conform decks supporting concrete stoop slabs.

For state confirm data supporting interfor for visital attach dack to supporting members with 51° a pudds welds per the following planm injust.
 For state contemport, a supporting members with 51° a pudds welds per the 10 Contemport, a support of the support of the support of the support 13 Contemport, a support of the support 13 Contemport, a support of the support

2. All steel deck shall span a minimum of three spans, unless otherwise approved.

REACTIONS ARE VERTICAL AND ARE TO BE CONSIDERED IN THE UP AND DOWN DIRECTION REACTIONS ARE EQUIVALENT STATIC REACTIONS REPORTED AS ULTIMAT FOR CONNECTION DESIGN PER LRFD. IF ASD IS USED, DIVIDE THE REACTI YA FACTOR OF 1.5. USE PROPER MATERIAL-AND LIMIT-STATE-SPECIFIC REDUCTION FACTOR (& OR OF PER CODE COR CONNECTION DESIGN.

Office of

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and Facilities

Management

VA U.S. Department

Affairs

GENERAL NOTES

1 3 4 7 1 8 2 _____ 5 6 1 9 10

Verification and Inspection

Inspection of reinforcing steel, including prestressing tendons, and placement.

Inspection of anchors post-installed in hardened concrete members (b).

Verifying use of required design mix.

Inspection for maintenance of specific

a. Application of prestressing forces

sed concrete

inspection of presi

For St 1 inch=25.4mm

а.

Exceptions:

Inspection of reinforcing steel welding in accordance with Table 1705.2.2, item 2b.

Inspection of anchors cast in concrete where allowable loads have been increased or where strength design is used.

At the time fresh concrete is sampled to fabricate specimens for strength tests, perform shamp and air content tests, and determine the temperature of the

Verification of in-situ concrete strength, prior to stressing of tendons in post-tensioned concrete and prior to removal of shores and forms from beams and

Inspect formwork for shape, location and dimensions of the concrete member being formed.

STATEMENT OF SPECIAL INSPECTION:

Special inspections and Testing requirements per Chapter 17 of the IBC in addition to Section 110 of the IBC (Inspecti the Building Official). See Specs. for additional information.

Structural Testing & Special Inspection Program Summary Schedule Material Type of Report Frequency IBC Section

 1704.2.5
 Shop Febrication
 SI-8
 Upon Completion

 1705.2
 Steel
 SI-8
 Weekly
 1705.3 Concrete SI-S Weekly 1705.6 Soils/Earthwork SI-T Upon Completion 1705.7-1705.9 Deep Foundation SI-T Continuous

SI-S Special Inspector-Structural SI-T=Special Inspector-Technical

1703.1 APPROVALS 1. Agency must be approved by the Building Official or AHJ.

2. Agency must be independent of the contractor responsible for work and disclose possible conflicts of interests

1704.2.4 - SPECIAL INSPECTOR RESPONSIBILITIES

- Submit inspection reports to the Building Official, Architect, Engineer of Record (EOR), and Contractor, stating th or was not in conformance with construction documents.
- Discrepancies shall be brought to the immediate attention of the contractor for correction
- If discrepancy is not corrected, it shall be brought to the attention of the building official and EOR in a timely ma remediation or acceptance prior to the completion of work.
- 4. Submit a final report documenting required special inspections and correction of any discrepancies noted.

1704.2.5 - FABRICATION:

- Where fabrication of structural members and assemblies are being fabricated on the premises of a fabricator's shop, special inspection is required of the fabricated term.

Note: Where Special Inspection and Testing of Shop Fabricated Components is required, it shall conform to the Special Inspection and Testing required in the field for the material specific section the component is fabricated from.

Exception: Special Inspection of the Fabricator's shop is not required if approved per Section 1704.2.5.2.

	TABLE 1705.6 REQUIRED VERIFICATI	ON AND INSPECTION	SOILS
	Verification and Inspection Task	Continuous During Task Listed	Periodically During Task Listed
1.	Continuous inspections shall be performed during installation of pile foundations and shall include equipment used, pile dimensionst, tip elevations, final depth, final installation torque, and other performent installation data.	х	
2.	Verify excavations are extended to proper depth and have reached proper material.	-	×
3.	Perform classification and testing of controlled fill materials.	-	х
4.	Verify use of proper materials, densities and lift thicknesses during placement and compaction of controlled fill.	х	
5.	Prior to placement of compacted fill, observe subgrade and verify that site has been prepared properly.	-	x

	TABLE 1705.7 REQUIRED VERIFICATION AND INSPECTION OF DEEP FOUNDATION ELEMENTS									
Ver	ification and Inspection Task	Continuous During Task Listed	Periodically During Task Listed							
1.	Verify element materials, sizes and lengths comply with the requirements.	х	-							
2.	Determine capacities of test elements and conduct additional load tests, as required.	x	-							
3.	Observe driving operations and maintain complete and accurate records for each element.	х	-							
4.	Verify placement locations and plumbness, confirm type and size of harment, record number of blows per foot of penetration, detormine required penetrations is achieve design capacity, record tip and but elevations and document any diamage to boundation element.	x	-							
5.	For steel elements, perform additional inspections in accordance with Section 1705.2.		-							
6.	For concrete elements and concrete-filled elements, perform additional inspections in accordance with Section 1705.3.		-							
7.	For specialty elements, perform additional inspections as determined by the registered design professional in responsible charge.	-	-							

Γ	Verification and Inspection Task	Continuous	Periodic
1.	Continuous inspections shall be performed during installation of pile foundations and shall include equipment used, pile dimensions, tip elevations, final depth, final installation torque, and other performent installation data.	x	-

weiding inspection Tasks	
Welding procedure specifications (WPSs) available	Р
Manufacturer certifications for welding consumables available	Р
Material identification (type/grade)	0
Welder identifications system (1)	0
Fit-up of groove welds (including joint geometry)	0
Configuration and finish of access holes	0
Fit-up of filet welds	0
Check welding equipment	
Use of qualified welders	0
Control and handling of welding consumables	0
No welding over cracked tack welds	0
Environmental conditions	0
WPS followed	0
Welding techniques	0
Welds cleaned	0
Size, length and location of welds	Р
Welds meet visual acceptance criteria	Р
Arc strikes	Р
k-area (2)	Р
Backing removed and weld tabs removed (if required)	Р
Repair activities	Р
Document acceptance or rejection of welded joint or member	Р
(1) The fabricator or erector, as applicable, shall maintain a system by whic welded a joint or member can be identified. Stamps, if used, shall be the low	
(2) When welding of double plates, continuity plates or stiffeners has been p area, visually inspect the web k-area for cracks within 3 in. (75mm) of the we	
0 - Observe these items on a random basis. Operations need not be delaye	d pending these

P. Barform there tasks for each walded joint or member N5.5

Non-destructive Testing (NDT) of Welds shall be performed in accordance with AWS D1.1/D1.1M based on the following criteria: For structures in Risk Category II or IV, Ultrasonic Testing (UT) shall be performed o all Complete Joint Penetration (CJP) groove welds for materials 5/16* thick or greater Structures in Risk Category II, UT shall be performed on 10% of CJP groove welds for materials 5/16" thick or greater. When flange or web thickness exceeds 2°, thermally cut access holes shall be tester using Magnetic Particle Testing (MT) or Penetrant Testing (PT), any crack is unacceptable. Welded joints requiring soundness per Appendix 3, Table A-3.1 shall be tested by Radiographic Testing (RT) or UT. Reduction in the rate of UT is prohibited. Reduction rate for UT - Where the initial rate for UT is 100%, the NDT rate for an individual welder is permitted to be reduced to 25% provided the reject rate is 5% or less based on a minimum of 40 welds tested. For continuous welds over 3°, each 12 increment shall be considered on weld.

Increase rate for UT - Where the initial rate for UT is 10%, the NDT rate for an individual welder shall be increased to 100% if the reject rate is over 5% based minimum of 20 welds tested. Rate may be reduced if reduction rate criterion is

2	Inspection of welding:										
	а.	Cold	formed steel deck:								
		1.	Floor and roof deck welds	-	х	AWS D1.3					
	b. Reinforcing steel:										
		1.	Verification of weldability of reinforcing steel other than ASTM A706	-	×						
		2.	Reinforcing steel resisting flexural and axial forces in intermediate and special moment frames, and boundary elements of special structural walls of concrete and shear reinforcement.	×	×	AWS D1.4 ACI 318: Section 3.5.2					
		3.	Shear reinforcement	х	-						
		4.	Other reinforcing steel	-	х						
(a). V	a). Where applicable, see also Section 17.05.11, Special inspections for seismic resistance.										

TABLE 1705.2.2 REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL

Continuous Periodic Referenced Standard (a)

Applicable ASTM material standards

с

n

F

OPEN-WEB STE	EL JOISTS		
Verification and Inspection	Continuous	Periodic	IBC Reference
Verify size and grade of joists upon placement	-	х	-
After setting, verify proper bearing length is provided	-	х	-

applicable, see also Section 17.05.11, Special inspections for seismic r	esistance.		
OPEN-WEB STE	EL JOISTS		
and Inspection	Continuous	Periodic	IB

Bolting Inspection Tasks		
Manufacturer's certifications available for fastener materials	Р	
Fasteners marked in accordance with ASTM requirements	0	
Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)	0	
Proper boiling procedure selected for joint detail	0	
Connecting elements, including the appropriate faying surface condition and hole preparation, if specified, meet applicable requirements	0	
Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used	0	
Proper storage provided for bolts, nuts, washers and other fastener components	0	
Fastener assemblies, of suitable condition, placed in all holes and washers (if required) are positioned as required.	0	
Joint brought to the snug-light condition prior to the pretensioning operation	0	
Fastener component not turned by the wrench prevented from rotating	0	
Fasteners are pretensioned in accordance with the RCSC Specification, progressing systematically from the most rigid point toward the free edges.	0	
Document acceptance or rejection of bolted connections	Р	
 Observe these items on a random basis. Operations need not be delayed pending these inspections. 		

P - Perform these tasks for each welded joint or steel

N5.7

TABLE N6.1 Inspection of Steel Elements of Composite C Placement

Р ment acceptance or rejection of steel elements

0 - Observe these items on a random basis. Operations ne inspections. P - Perform these tasks for each steel member

awing Titl oject Titl oject Numbe CONSULTANT ARCHITECT/ENGINEER OF RECORD STAMP Office of 438-460 TESTING SCHEDULES CONSTRUCT NEW SPS BID DOCUMENTS Construction Building Number and Facilities 5 ANDERSON ALC. No 11128 MICHAEL P. MERRON uina Numbe Management ocation Sioux Falls, SD. FULLY SPRINKLERED 18605 Ter Ave. N. 4180 Flymourh, MN 95441 F 763.412.4080 | F 789.412.4090 | Johanni.com Anderson Registerring of Minetenza, MC | Pinj # 16584 IChecked Drawn VA U.S. Department of Veterans Affairs le eue Date SG001 2-14-20 02/14/2025 MPM/TGL MAQ Revisions: /A FORM 08-6231 Τ 2 4 5 6 1 1 8 9 3 1 7 10



uous Periodic Referenced Standard (a)

x

Х

х

х

х

х

X ACI 318: 3.5, 7.1 - 7.7

AWS D1.4, ACI 318: 3.5.2

ACI 318: 8.1.3, 21.2.8

ACI 318: 3.8.6, 8.1.3, 21.2.8

ACI 318: Ch. 4, 5.2 - 5.4

ASTM C 172, ASTM C 31, ACI 318: 5.6, 5.8

ACI 318: 5.9, 5.10

ACI 318: 18.20 ACI 318: 18.18.4

ACI 318: Ch. 6.2

ACI 318: Ch. 6.1.1

ACI 318: 5.11 - 5.13

IBC Reference

1910.4

1908.5, 1909.1

1909.1

1910.10

1910.9

Verification and Inspection

Material verification of cold-1

Identification markings to conform to ASTM standards specified in the approved construction documents

b. Manufacturer's certified test reports

TABLE 1705.3 REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION

